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ESTIMATION OF GROUND MOTION PARAMETERS

Ву

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ABSTRACT

Strong motion data from western North America for earthquakes of magnitude greater than 5 are examined to provide the basis for estimating peak acceleration, velocity, displacement, and duration as a function of distance for three magnitude classes. Data from the San Fernando earthquake are examined to assess the effects of associated structures and of geologic site conditions on peak recorded motions. Small but statistically significant differences are observed in peak values of horizontal acceleration, velocity, and displacement recorded on soil at the base of small structures compared with values recorded at the base of large structures. Values of peak horizontal acceleration recorded at soil sites in the San Fernando earthquake are not significantly different from the values recorded at rock sites, but values of peak horizontal velocity and displacement are significantly greater at soil sites than at rock sites. Three recently published relationships for predicting peak horizontal acceleration are compared and discussed. Considerations are reviewed relevant to ground motion predictions at close distances where there are insufficient recorded data points.

INTRODUCTION

Peak horizontal acceleration is commonly used to scale response spectra or ground motion time histories for use in earthquake-resistant design, particularly in the case of nuclear power plant facilities (Newmark, Blume, and Kapur, 1973). Methods have also been proposed (Newmark and Hall, 1969) for constructing design spectra using three peak parameters, horizontal acceleration, velocity, and displacement, the advantage of using all three parameters being that together they convey some information concerning the shape of the spectrum as well as the amplitude level. In this report we present the analysis of a large number of earthquake data to provide the basis for estimating the peak acceleration, velocity, and displacement and duration of shaking for a hypothetical earthquake of a prescribed magnitude at a prescribed distance from the causative fault. This work is a continuation of that reported by Page and others (1972) and by Page and others (1975).

It is not our purpose to advocate the use of peak parameters in scaling design motions. We look forward ultimately to the development of new methods for prescribing design motions, methods more firmly based in the physics that governs faulting and wave propagation. Pending the development of such methods, we recognize widespread current practice and attempt to present the available strong motion data in a compact and useful form for estimating peak parameters.

DATA CHARACTERISTICS AND METHODS OF PRESENTATION

Sources of data. The data set includes 204 recordings from 19 earthquakes and is listed in Appendix B. The primary source of acceleration data is volume I of the series "Strong Motion Earthquake Accelerograms" published under the

direction of D. E. Hudson by the Earthquake Engineering Research Laboratory of the California Institute of Technology; values of velocity, displacement, and duration came from volume II of the same series. We used volume I for acceleration because volume II gives data at equal time intervals of 0.02 sec. and that tends to bias the peak acceleration toward lower values. A few of the acceleration data came from other sources listed in Appendix B, principally <u>U.S. Earthquakes</u>, an annual publication of the U.S. Department of Commerce.

Distances. In all cases the distance used is the shortest distance between the surface of fault slippage and the recording point. This would clearly be the preferred measure of distance if radiation were uniform over the surface and if the surface were known. The second condition is sometimes not met; the first is probably never met. Other measures of distance have been used in strong motion data analysis, particularly, epicentral distance, hypocentral distance, and distance from the center of energy release. The use of epicentral distance or hypocentral distance has the advantage that they are more commonly known and special studies are not required to determine them. In some cases, however, these measures are clearly inappropriate, as in the case of a long fault rupture with epicenter at one end and recording stations at the other. The Parkfield, California, earthquake of 1966 provides an example of such a situation. The use of distance to the center of energy release is a way of avoiding the assumption of uniform radiation over the rupture surface, but in the case of long ruptures this measure, too, may be inappropriate. In our opinion the best choice for general purposes is the closest distance to the rupture surface, but the uncertainties resulting from nonuniform radiation over the surface should be kept in mind. An illustration of those uncertainties is provided by the Pacoima Dam recording of the San Fernando earthquake of 1971. On that record the source for the peak velocity and for the peak acceleration are different points on the fault, separated by perhaps 20 km, neither one of which is the closest point to the instrument (Hanks, 1974; Bouchen and Aki, 1977).

With a few exceptions the location of the rupture surface has been inferred from the aftershock distribution. In the case of the Imperial Valley, California, earthquake of 1940 the distance used is chosen in accordance with the interpretations of Richter (1958) and Trifunac and Brune (1970). In the case of the Hebgen Lake, Montana, earthquake of 1959, the distance used is the epicentral distance of the main shock, and in the case of the Puget Sound earthquake of 1949, the distance used is the hypocentral distance of the main shock assuming a minimum focal depth of 45 km. Sources of data used in estimating station distances are included in Table I.

In order to avoid obscuring the attenuation relationships we generally exclude data where the uncertainty in distance is large. Following Page and others (1972), we classify the distances as A, B, or C, according to the uncertainty (less than 2 km, 2 to 5 km, and 5 to 25 km, respectively). C quality data are only used in the case of the magnitude 7.1 Puget Sound earthquake and the magnitude 7.1 Hebgen Lake earthquake. In the plots to follow, the class A, B, or C is indicated by the size of the symbol, the largest for class A and the smallest for class C.

The assignment of distances in the case of the Parkfield earthquake deserves special mention. Originally it was believed that the rupture associated with the Parkfield earthquake extended along the San Andreas fault far enough to the southeast so that it passed within 80 meters of station

number 2 of the Cholame-Shandon array (Cloud and Perez, 1967). Lindh and Boore (1973), however, presented evidence that, at the time of the earthquake, no significant displacement occurred beyond a point 7 km northwest of station number 2. Modeling studies by Trifunac and Udwadia (1975) tend to confirm the Lindh and Boore interpretation and we follow it in this report.

Classification of data. We have divided the data into classes in accordance with magnitude, site geology, and size of associated structure. The data is divided into three magnitude classes (5.0-5.9, 6.0-6.9, and 7.0-7.9) on the basis of the Richter local magnitude (Richter, 1958), if available, otherwise surface wave magnitude is used. Sources of data for assigning magnitudes are included in Table I. The Imperial Valley earthquake is assigned a magnitude of 6.4 in accordance with a determination by Trifunac and Brune (1970) and in contrast with the value 7.1 that is commonly given.

Kanamori and Jennings (written communication) have recently developed a method of determining Richter local magnitude from strong motion records.

Their magnitude assignments are in general agreement with ours. The largest difference is for the Puget Sound earthquake of 1949 for which their value is 6.5 in contrast with our value of 7.1.

We assign recording sites to one of two categories, "rock" or "soil" by applying our best judgment to the available site descriptions. We assign stations to the rock category if they are underlain by material described by such terms as "granite", "diorite", "gneiss", "chert", "graywacke", "limestone", "sandstone", "siltstone", or "shale". Stations are assigned to the soil category if they are underlain by sufficient thickness of material described by such terms as "alluvium", "sand", "gravel", "clay", "silt", "mud", "fill", or "glacial outwash". If we judge from the site description

that soil material overlying rock is less than 4 to 5 meters thick we ignore it. Sources for site descriptions are given in Appendix B. The reader should be warned that considerable uncertainty and ambiguity attends the geological classification of recording sites. We do not, however, even suggest conclusions that rely on the validity of the classification of a single station. We are concerned only with trends revealed by comparing whole classes of data.

Much of the data comes from the basements or ground floors of buildings or from the abutments of dams. In the analysis of strong motion data it is commonly assumed that the influence of the structure on the motion of the base can be ignored and that the data as recorded represent free-field ground motion. We have attempted a limited test of this assumption by classifying recording sites in accordance with the size of the associated structure; class 1 for sites at the base of one- or two-story buildings and class 2 for sites at the base of taller buildings or on dam abutments. Comparison of the two classes using data from the San Fernando earthquake is described in a subsequent section. Briefly, there are small but statistically significant differences.

In the case of velocity and displacement one would expect the data from small structures to be more representative of free-field motion. The transfer functions relating motion at the base of structures to free-field motion tend toward unity for frequencies small compared to the fixed-base natural frequencies of the structure. (For examples of theoretical and empirical transfer functions see Duke and others, 1970, and Crouse and Jennings, 1975). The small structures have natural frequencies mostly in the range of 2 to 10 Hertz, which is significantly above the range of frequencies dominant in the

velocity and displacement time histories. The case of acceleration is more complicated. The natural frequencies of the small structures are in the same range as the frequencies dominant in the acceleration time histories, and the effect of the structure may be to raise or lower the acceleration depending on the spectrum of the earthquake and the details of the transfer function. We still believe, however, that the smaller structures provide the better basis for estimating free-field motion, even for acceleration. The reason is that the transfer functions tend to fall below unity for frequencies substantially greater than the natural frequencies of the structures. In some cases, such as the observed transfer function for the Hollywood Storage Building, the attenuation at high frequencies is large. So, we would expect the acceleration values for the large structures to be systematically biased downward. In fact, the comparison of San Fernando data shows smaller accelerations on the average for the large structures. Our main emphasis, therefore, is placed on the data from the small structures, but for the horizontal component data we also provide plots and regression parameters for the whole data set.

Geographical distribution. In an attempt to keep the data sample reasonably homogeneous, only records obtained in the western part of North America were included. In order to avoid bias from the extremely dense cluster of instruments in downtown Los Angeles a special selection procedure was used in the area between 34.00° and 34.11° North Latitude and 118.24° and 118.45° West Longitude. Within each of the two geological site categories only one recording per earthquake was allowed for each structure category, making a maximum of four possible recordings from the designated area for one earthquake. Selection was made by choosing the station with the smallest

identification number of all the eligible stations. In Appendix B stations so chosen are denoted by an asterisk.

Presentation of data. Peak horizontal acceleration, velocity, and displacement data are plotted against distance on log-log grids for each magnitude class. The peak values for horizontal motion are taken from the component with the larger peak. Duration values are plotted against distance on a linear grid. The measure of duration used is the time interval between the first and last horizontal acceleration peaks equal to or greater than 0.05 g. The value is taken from the horizontal component that gives the larger value. This is the definition used by Page and others (1972). It is a relatively crude measure, but it is simple to determine and is of some value in characterizing ground motion. Peak vertical acceleration, velocity, and displacement are plotted on log-log grids in the same way as the horizontal data.

Statistics. The nature of the strong motion data set is not such as to bear the weight of elaborate or subtle statistical inferences. For that reason we emphasize plots showing the individual data points. We do, however, indulge in statistical analysis to the extent of determining least-squares straight lines relating the logarithm of the peak parameters to the logarithm of distance and determining the confidence limits for the prediction of a single value of the dependant variable (Dixon and Massey, 1957).

We have attempted to avoid bias in the regression analysis by not including points that are either too close or too far from the fault. In the first case the data are too sparse to indicate the proper functional form for the regression and in the second the data set is incomplete because not all instruments were triggered by the motion. For small structures the data used

in our regression calculations are contained within the ranges 5-30, 15-55, and 40-150 km for magnitude classes 5.0-5.9, 6.0-6.9, and 7.0-7.9, respectively. For the San Fernando earthquake the range is 15-100 km. For the whole data set including both large and small structures the ranges are the same as for the small structures except for magnitude class 6.0-6.9 for which the range is 10-55 km.

The straight lines obviously fit the data as well as would any simple relationship. Curvature that might be caused by anelastic attenuation is completely obscured by the scatter in the data.

The scatter is approximately constant independent of distance. This suggests that the decision was correct to fit a straight line relationship to the logarithms of variables rather than fit a power law relationship to the variables themselves.

ALL EARTHQUAKES

General comments. Data for all the earthquakes is presented in this section, with emphasis on the data from small structures because, for reasons given previously, we consider those data a better guide to free-field motion. In the succeeding section data from the San Fernando earthquake are examined to assess the effect of structure and the effect of local site geology.

Horizontal acceleration. Peak horizontal acceleration data from the small structures for the three magnitude classes are shown in Figures 1, 2, and 3. The relations among the magnitude classes are summarized in Figure 4, which shows the overlap of the 70 percent prediction intervals. The accelerations clearly increase with magnitude in those distance ranges for which there is overlap between the classes. The scatter for the magnitude 5.0-5.7 data is

significantly greater than that for either of the other two classes. This may in part be due to the fact that a number of different earthquakes contribute substantially to the data set for the 5.0-5.7 class, whereas the 6.0-6.4 class is dominated by data from the 1971 San Fernando earthquake and the 7.1-7.7 class is dominated by data from the 1952 Kern County earthquake.

The rate of attenuation of acceleration with distance for the magnitude 5.0-5.7 class appears to be greater than indicated by the slope of -0.9 for the mean regression line in Figure 1. This is suggested by the systematic tendency for the data points at distances beyond 30 km to lie below an extension of the mean regression line. Data beyond 30 km are excluded to avoid bias toward larger values because ground motions in this distance range are not always sufficient to trigger the existing accelerographs. In this case, however, the data points beyond 30 km lie below not above the mean regression line. The distance range for which a reasonably complete data set is currently available is not adequate for a good determination of slope; the standard error of the slope for the magnitude 5.0-5.7 class is 0.5. Judging from the data at greater distances, the slope of -1.2 ± 0.3 for the mean line for the magnitude 6.0-6.4 class appears to be a better estimate of the rate of attenuation to distances of at least 100 km for that data set. The slope of -2.0 ± 0.4 for the magnitude 7.1-7.7 class may overestimate the rate of attenuation, but the data are scanty.

Horizontal velocity. The peak horizontal velocity data from the small structures for the three magnitude classes are presented in Figures 5, 6, and 7. There are fewer velocity than acceleration points because integrations were not available for all the accelerograms. There are so few points for the magnitude 7.1-7.7 class that regression lines are not included on Figure 7.

As with acceleration, the peak velocity at a given distance tends to increase with magnitude. This is illustrated in Figure 8, which gives the 70 percent prediction intervals for the three magnitude classes. The interval for the 7.1-7.7 class is shown by dashed lines to emphasize the uncertainty in slope.

The slope of -0.6 ± 0.4 for the mean regression lines for the magnitude 6.4 data appears to be an underestimate of the rate of attenuation if one considers the San Fernando data described in the next section, which give better determinations because the distance range extends to 100 km. We were confident that all the instruments out to 100 km triggered in the San Fernando earthquake, but that was not the case for the whole magnitude class. Horizontal displacement. The peak horizontal displacements for the three magnitude classes are given in Figures 9, 10, and 11. The scatter of the data is larger than for acceleration or velocity in each magnitude class, and the standard errors of the slopes of the mean regression lines exceed 0.5. The displacements are derived from double integration of high-pass filtered accelerograms and therefore represent high-pass filtered versions of the true ground displacement. The longer periods, which are contaminated by processing noise, are removed.

Hanks (1975) has studied the errors in displacement records derived by double integration of filtered accelerograms. He finds that the errors are typically less than 1 cm in the period range 5-8 seconds, 1-2 cm at periods near 10 seconds, and 2-4 cm in the period range 10-15 seconds. This raises the possibility that some of the low-amplitude data points in Figures 9 and 10 may be influenced by noise and may represent upper bounds to the actual ground displacement. Examination of the displacement records reveals that some of the low amplitude records have a character that is suggestive of noise rather

than signal. In spite of this, we have proceeded in the analysis with the understanding that the results may be compromised to some extent by the effect of noise on the weaker motions.

The overlap of the 70 percent prediction intervals for the three magnitude classes is shown in Figure 12. The amplitude increases with magnitude.

<u>Duration</u>. All the horizontal duration data are plotted in Figure 13 with different symbols for the different magnitude classes. The "X" symbol in Figure 13 denotes a zero duration; in such a case the peak acceleration on the record is less than 0.05 g. The upper and lower rows of X's represent zero durations for magnitude classes 6.4 and 5.3-5.7, respectively.

Two obvious and expected features stand out in Figure 13: the durations increase with increasing magnitude, and they decrease with increasing distance. The influence of magnitude is a reflection of the larger fault size and consequent increased time of rupture as magnitude is increased. The effect of distance is the result of the general decrease in amplitude with distance, given that we have used a fixed amplitude in the definition of duration. Had we defined duration in terms of some fraction of the peak amplitude, it is likely that the spreading apart of the seismic phases would have led to an increase of duration with distance.

<u>Vertical data</u>. The vertical data are presented in the same manner as the horizontal data. Peak vertical accelerations for the three magnitude classes are shown in Figures 14, 15, and 16; peak vertical velocities are shown in Figures 17, 18, and 19; and peak vertical displacements are shown in Figures 20, 21, and 22.

The whole data set. For the horizontal components, data from both large and

small structures taken together are presented in Figures 23 through 31.

THE SAN FERNANDO EARTHQUAKE

General comments. The San Fernando earthquake supplied more than one quarter of the total data points in our sample. The large number of data points from a single event provides the best basis for examining the effect of structure and local site conditions. The San Fernando earthquake also gives more accurate values than the whole M = 6.0-6.9 data set for the slopes of the regression lines for peak parameters against distance. This is the case because, as previously mentioned, the statistical analysis can be carried out over a greater range of distance for the San Fernando earthquake. The reader is reminded that, to avoid bias, not all the records from downtown Los Angeles are included in the data set.

In comparing peak parameters for different structural types and site conditions we use an analysis of variance technique (Acton, 1959, p. 80-83) to test the statistical significance of the observed differences between one data set and another. To state the matter more precisely, we consider the variance of the residuals and examine the statistical significance of the reduction in variance that occurs when different regression lines are fit to the two different data sets. The technique allows us to break down the reduction of variance into a component attributable to separate slopes and a component attributable to separate means. In what follows when we say a difference is significant we mean that it corresponds to a significant reduction in the variance of the residuals. A word of caution is appropriate concerning the analysis of variance tests. Essentially, they enable us to see how the differences between data sets compare with those that might be caused by

random sampling error. We should not be confident, however, that the strong motion data sets represent random samples, and in any case the statistical tests say nothing about the real physical meaning of the differences between data sets.

The effect of structure. In Figure 32 comparison is made between peak horizontal acceleration values recorded on soil at the base of small structures (S1) and large structures (S2). Figure 33 shows the mean regression lines and the 70 percent prediction intervals determined for the Sl and S2 data separately. The mean regression line for the S1 data lies above that for the S2 data and the analysis of variance tests indicate that the difference is significant at the 90 percent level. The difference in slope is not significant. The same comparisons are made for horizontal velocity in Figures 34 and 35. In the case of velocity the mean regression line for the SI data lies generally below that for the S2 data, though they cross, and the difference is statistically significant at the 98 percent level, though unimpressive to the eye. The S1 line is steeper, and the difference in slope is significant at the 90 percent level. The horizontal displacement data is given in Figures 36 and 37. For displacement the mean regression line for the S1 data lies below that for the S2 data, and the difference is significant at the 99 percent level. The difference in slope is not significant.

In summary we can say that for most of the distance range covered by the regression analysis peak horizontal acceleration is less and peak horizontal velocity and displacement are greater on the average at the base of large structures than at the base of small structures. The attenuation with distance is greater for the small structures for all three parameters, but the difference is statistically significant only in the case of peak velocity.

The result that acceleration values from the large structures are lower on the average is what would be expected if soil-structure interaction biases those data downward. This encourages us in our preference for the small-structure data as a basis for estimating free-field ground motion. In general, however, the differences between the data from the large structures and the small structures are relatively small compared with the range of either data set, and we do not believe that firm conclusions are warranted solely on the basis of formal statistical tests. The differences may be due to soil-structure interaction, but more study would be required to demonstrate this. The effect of site geology. Figure 38 gives a comparison of peak horizontal acceleration recorded at the base of small structures on rock and soil. Figure 39 shows the mean regression line and 70 percent confidence intervals determined for the two data sets separately. The analysis of variance tests indicate that the differences are not significant in either slope or level. Peak horizontal velocity data for small structures on both rock and soil sites are compared in Figures 40 and 41. The mean regression line is higher for soil and that difference is significant at the 98 percent level. The difference in slope is not significant. Peak horizontal displacement data are compared in Figures 42 and 43. The mean regression line for soil is higher and the difference is significant at the 98 percent level. The difference in slope is not significant even at the 75 percent level.

Apparently, peak horizontal acceleration is essentially the same on the average on rock and soil sites, whereas peak horizontal velocity and displacement are both larger on soil sites. This relationship is not the result of any obvious bias in the data. Examination of Figures 38, 40, and 42 does not show any gross effect from bias in the distribution of stations with

distance. To test for bias due to the non-uniform azimuthal distribution of the data (Hanks, 1975) we determined the azimuth of each station with respect to a point in the center of the zone of fault rupture (34.37° N. Lat., 118.42° W. Long.). A mean regression line against distance was determined for all the peak horizontal acceleration data for small structures in the distance range 15-100 km (with distance measured to the closest point on the rupture surface as before). Residuals to that regression line are plotted against azimuth in a polar diagram in Figure 44 with rock sites shown as "X"s and soil sites as diamonds. The circle represents zero residual. No strong systematic difference is apparent between rock and soil. Figure 45 gives the corresponding plot for the velocity data. Although the azimuthal coverage is far from complete, we can say that in any range of azimuth for which both rock and soil points are present, the soil residuals are more positive. Similar results are obtained for the displacement data (Figure 46).

We tentatively conclude that amplification of velocity and displacement is a real effect associated with soil sites. We presume that for the soil sites some sort of amplification mechanisms are operating on the longer periods that are dominant on velocity and displacement records. In the case of the shorter periods that are dominant on acceleration records these mechanisms are counterbalanced by anelastic attenuation. We will not speculate here on the nature of the amplification mechanisms. Similar conclusions on the effect of site conditions on strong motion in the San Fernando earthquake were reported by Duke and others (1972), Trifunac (1976), and Arnold and others (1976).

PREVIOUSLY PUBLISHED CURVES FOR PEAK ACCELERATION

There are a large number of published correlations between ground motion parameters and distance, magnitude and site conditions. They have been described by Trifunac and Brady (1976) and discussed by Seed and others (1976). We consider here only three recently published, widely known relationships proposed for peak acceleration.

All studies of strong motion data are handicapped by the limited number of data points at small distances from the source. Attempts to predict strong motion parameters at short distance are forced to rely upon rather tenuous assumptions.

Curves for mean peak acceleration are shown in Figure 47 for a magnitude 6.6 earthquake. Also shown (solid lines) is the 70 percent prediction interval for the small-structure, magnitude 6.0-6.4 data set of this report. Most of the points in that data set came from the magnitude 6.4 San Fernando earthquake, so the comparison is appropriate from the standpoint of magnitude. Data from large structures, however, were not excluded in the development of the other curves.

The curve labeled "S" was developed by Schnabel and Seed (1973) for rock sites and is based on strong motion data extended to near distances with the help of theoretical attenuation curves. Because the theoretical curves are based on the conservation of radiated energy, however, they apply strictly only to quantities related to the energy represented by the whole duration of the seismic record. Application of the curves to peak parameters is an approximation of uncertain accuracy. The measure of distance used by Schnabel and Seed is the shortest distance to the rupture surface, the same measure as

used in this report.

The curves labeled "TO" and "T2" are the mean curves given by Trifunac (1976) for soft and hard sites, respectively. These curves are based on a data set very similar to the one used in this report, including data from both large and small structures. The distance measure used by Trifunac is epicentral distance. The curves were fitted to the data on the assumption that the distance dependence is that of the function given by Richter (1958) for calculating local magnitudes in Southern California. The accuracy of that assumption is difficult to evaluate. Furthermore, the distance function given by Richter is not very well defined for distances between 0 and 20 km, which is the range most important for strong motion predictions.

The curve labeled "D" was developed by Donovan (1973) for soil sites. It was obtained by fitting 678 data points by a function of the form

$$y = b_1 e^{b_2 m} (R + 25)^{-b_3}$$

where y is peak acceleration, m is magnitude, R is hypocentral distance in kilometers and b_1 , b_2 , and b_3 are adjustable constants. The arbitrary constant 25 that is added to the distance is for the purpose of reducing the predicted values at small distances. The size of the constant has a very large influence on the near values, but sufficient near data points are not available for a meaningful determination of the appropriate size. Donovan states that the function fits the data better when the arbitrary constant is 25 than when it is zero, but it is unclear why it should be 25 rather than 15, 10, or 5.

The corresponding curves are compared in Figure 48 for a magnitude 7.6

earthquake. The solid lines show the 70 percent prediction interval for the magnitude 7.1-7.7 data set of this report. Most of the points in that data set came from the magnitude 7.7 Kern County earthquake.

The amount of disagreement shown in Figures 47 and 48 is not surprising in view of the different assumptions, different measures of distance, and different data sets used in arriving at the different curves. The disagreement is, as might be expected, the greatest at short distances.

ESTIMATION OF PEAK PARAMETERS AT SHORT DISTANCES

General comments. The regression lines given in a previous section of this report provide the means for estimating peak ground motion parameters at distances greater than 5 km for magnitude 5.0-5.9 earthquakes, at distances greater than 15 km for magnitude 6.0-6.9 earthquakes and at distances greater than 40 km for magnitude 7.0-7.9 earthquakes. Unfortunately, most of the damage from earthquakes can be expected to occur at shorter distances. Attempts have been made, as described in the preceding section, to provide curves for estimating at shorter distances. For reasons given in the preceding section we do not have complete confidence in those curves. We will not venture our own set of curves, but will discuss briefly some of the considerations bearing on ground motion estimates near the source. Further discussion of these questions in greater depth is given by Boore (1974).

There have been a number of studies using simplified models of the faulting process to set limits on the ground motion at the fault surface (Housner, 1965; Ambraseys, 1969; Brune, 1970; Ida, 1973). Brune's (1970) near source model assumes that rupture occurs instantaneously over the fault

plane. The peak particle velocity is proportional to the stress drop and equals 100 cm/sec for a stress drop of 100 bars. The peak acceleration is infinite if all frequencies are included, but if frequencies above 10 Hz are filtered out of the acceleration pulse the peak value is 2 g. This is a useful model for relating ground motion to the physics of the rupture process, but it does not give firm upper limits. An argument can be made for larger motions if one takes rupture propagation into account (Ida, 1973; Andrews, 1976). Furthermore, the peak values of ground motion may represent localized high stress drops as Hanks and Johnson (1976) have suggested for peak acceleration. Such localized stress drops might easily exceed one kilobar.

The peak acceleration at the surface is limited by the strength of near surface materials as has been pointed out by Ambrasey (1974). For sites near the source underlain by soil material of low strength, this factor may control the value of peak acceleration. This consideration may also apply to rock sites if the rock is sufficiently weathered. Determination of the limiting acceleration, however, would require reliable measurement of the dynamic, in situ strength of the soil at a particular site. In the absence of adequate measurements one must presume that the acceleration could be at least as large as 0.5g, which was recorded on a thickness of more than 60 meters of water-saturated alluvium at station number 2 in the Parkfield earthquake (Shannon and Wilson, Inc. and Agbabian Associates, 1976).

In the case of peak displacement, as pointed out by Trifunac (1976), if one assumes no overshoot, the peak is limited to less than one half the static dislocation amplitude. The latter is known for many historical earthquakes and may be estimated as a function of magnitude (Bonilla and Buchanan, 1970).

The accelerogram recorded at Pacoima Dam during the San Fernando

earthquake has major significance for near source ground motion estimates. The instrument is located only 3 km from the rupture surface at a rock site where the topographic relief is severe. The peak recorded horizontal acceleration is 1.25q, velocity 113 cm/sec, and displacement 38 cm. This is the only accelerogram ever recorded within 5 km for an earthquake of magnitude as large as 6.4, and as such ought to have strong influence on estimates of near-source ground motion. The possibility of topographic amplification needs consideration. A two-dimensional finite-difference study by Boore (1973) suggests that the acceleration may have been amplified by as much as 50 percent but that the velocity and displacement were relatively unaffected. Given these considerations, it would be difficult for us to accept estimates less than about 0.8g, 110 cm/sec, and 40 cm, respectively, for the mean values of peak acceleration, velocity and displacement at rock sites within 5 km of fault rupture in a magnitude 6.5 earthquake. We recognize that these numbers represent one earthquake with a particular focal mechanism and that estimates are bound to change when more data becomes available. We presume that the statistical scatter about the mean will be at least as great for the near-in sites as at the greater distances where data is available.

The accelerograph at Pacoima dam was only 3 km from the nearest point on the rupture surface, but the nearest point was not the source of the peak motions. As noted previously the source for the peak velocity and for the peak acceleration are different points on the rupture surface separated by perhaps as much as 20 km (Hanks, 1974; Bouchon and Aki, 1977).

Above magnitude 6.5 there are essentially no data for estimating the effect of magnitude on near-fault peak acceleration, velocity and displacement, other than the static fault offset divided by 2 as a bound on

the peak displacement. Conservatism requires the presumption of some increase with magnitude. Hanks and Johnson (1976) presented a set of peak acceleration data at source distance of approximately 10 km for earthquakes in the magnitude range 3.2-7.1. The only data point above magnitude 6.5 was for the Imperial Valley earthquake of 1940 which they assign a magnitude of 7.1 in contrast to our value 6.4, so the data set can be applied to magnitudes greater than 6.5 only as an extrapolation. The data set shows some dependence of peak accelerations on magnitude, but Hanks and Johnson argue that the data are consistent with the idea of magnitude-independent source properties. The data plotted as the logarithm of peak acceleration against magnitude can be fit by a straight line with a slope equivalent to an increase by a factor of 1.4 per magnitude unit. This should not be used for extrapolation beyond magnitude 6.5, however, because the data set was deliberately chosen to represent relatively high values, and thus the slope of the line fitting the data may not be the same as the slope of the line representing mean values or, for that matter, of the line representing values for any fixed probability.

At sites other than rock sites accelerations might be less because of the limited strength of near-surface materials, but, as previously noted, determining how much less would require dynamic, <u>in-situ</u> measurements of soil properties. The amplification of peak velocity at soil sites compared to rock sites may not be so great close to the fault because of the energy lost in nonlinear soil deformation, but numerical modeling (Joyner and Chen, 1975) demonstrates the possibility of amplification of velocity by as much as 30 percent even under conditions of intense deformation. The possibility of greater amplification cannot be excluded. Amplification of displacement at soil sites should be expected close to the fault, as at greater distances, if

the soil column is sufficiently thick.

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TABLE I
Sources of Data Used in Assigning Magnitudes and Station Distances

Earthquake		te (GM n Day		Sources
Bear Valley, California	2	- 24	72	Ellsworth(1975)
Bear Valley, California	11	28	74	Person(1975);W.H.K. Lee (written communication, 1976)
Ferndale, California	6	7	75	Nason and others(1975) Stewart Smith (written communication, 1976)
Daly City, California	3	22	57	Tocher(1959);Cloud(1959)
Lytle Creek, California	9	12	70	T.C. Hanks (written communication, 1971)
Parkfield, California	6	28	66	McEvilly and others(1967) Lindh and Boore(1973) Trifunac and Udwadia(1974) Lindh (oral cummunication, 1976)
Fairbanks, Alaska	6	21	67	Gedney and Berg(1969)
Santa Rosa, California	10	2	69	Steinbrugge and others(1970) Unger and Eaton(1970) Unger and Eaton (written communication, 1976)
Oroville, California	1	8	75	Bufe and others(1976) Lahr and others(1976)
Point Mugu, California	2	21	73	Ellsworth and others(1973) Boore and Stierman(1976) Stierman and Ellsworth(1976)
Managua, Nicaragua	12	23	72	Dewey and others(1973) Ward and others(1973) Knudson and Hansen A.(1973)
Imperial Valley, California	5	19	40	Trifunac and Brune(1970) Trifunac(1972);Richter(1958)

CONTINUED

TABLE 1 (CONTINUED)

Earthquake	Dat Month	e (GM Day		Sources
Borrego Mountain, California	4	9	68	Allen and Nordquist(1972) Hamilton(1972)
San Fernando, California	2	9	71	Allen and others(1973) Allen and others(1971) R.L. Wesson (written communication, 1974)
Puget Sound, Washington	4	13	49	Nuttli(1952);Page and others (1972)
Hebgen Lake, Montana	8	18	59	Tocher(1962); Page and others (1972)
Sitka, Alaska	7	30	72	Page and Gawthrop(1973) Gawthrop and Page (unpub- lished data)
Kern County, California	7	21	52	Gutenberg(1955) Richter(1955);Richter(1958) Page and others(1972)

APPENDIX A

STATISTICAL PARAMETERS

Linear regression analysis (Dixon and Massey, 1957) was employed to describe the distance dependence of the peak parameters. Using the symbol \underline{y} for the peak parameter and the symbol \underline{x} for distance we fit the data by a straight line

$$v = A + B u$$

where $v = \log_{10} y$

and $u = \log_{10} x$.

Values for A and B are given by the following equations

$$A = \sum_{\Sigma} v - B_{\Sigma} u$$

$$B = \frac{n \Sigma uv - \Sigma u \Sigma v}{n \Sigma u^2 - (\Sigma u)^2}$$

where the summations are taken over all the points in the data set and n is the number of points. The scatter in the data is measured by $s_{v|u}$, the standard error of estimate of \underline{v} for a given \underline{u} . That quanitity is obtained from the following equations:

$$s_{v|u} = \frac{n-1}{n-2} (s_v^2 - B s_u^2)$$

$$s_u^2 = \frac{1}{n(n-1)} [n \Sigma u^2 - (\Sigma u)^2]$$

$$s_{v}^{2} = \frac{1}{n (n-1)} [n \Sigma v^{2} - (\Sigma v)^{2}].$$

For a given confidence level, the prediction interval for a single prediction of v given u is

$$(A + Bu) + t_{\alpha/2, n-2} s_{v|u} \sqrt{1 + \frac{1}{n} + \frac{(u - \overline{u})^2}{(n - 1)s_u^2}}$$

where \overline{u} is the mean of u values, the confidence level is $(1-\alpha)$, and $t_{\alpha/2,n-2}$ is the abscissa of the Student's \underline{t} distribution for a cumulative probability of $(1-\alpha/2)$ and (n-2) degrees of freedom. The lines describing the prediction intervals are curved because of statistical uncertainty in the regression coefficient B. A measure of that uncertainty is the standard error of B, which is given by

$$s_B = s_V | u$$

$$s_V \sqrt{n-1}$$

Table Al lists the statistical parameters A, B, $\mathbf{s_{v|u}}$, $\mathbf{s_{B}}$ and n for the data sets discussed in the text. The number of the Figure displaying the data set is also given.

Table Al

Data Set	Figure No.	<u>A</u>	<u>B</u>	s _v j u	SB	n
HORIZONTAL ACCELER M = 5.0-5.7 class M = 6.0-6.4 class M = 7.1-7.7 class M = 5.0-5.7 all M = 6.0-6.4 all M = 7.1-7.7 all San Fernando R1 San Fernando S1 San Fernando S2	1 1 1 2	0.17 0.96 2.65 0.05 0.81 2.65 1.45 1.09 0.90	-0.93 -1.23 -2.01 -0.86 -1.20 -2.00 -1.56 -1.34 -1.29	0.37 0.20 0.26 0.35 0.20 0.21 0.18 0.18 0.15	0.46 0.32 0.43 0.40 0.15 0.31 0.23 0.25 0.15	19 16 9 24 44 14 10 12 18
HORIZONTAL VELOCIT M = 5.3-5.7 class M = 6.4 class 1 M = 7.1-7.7 class M = 5.3-5.7 all M = 6.4 all San Fernando R1 San Fernando S1 San Fernando S2	1 5 6	2.35 1.93 2.45 2.31 2.35 3.12 3.06 2.60	-1.22 -0.58 -0.72 -1.26 -0.85 -1.51 -1.31 -0.96	0.38 0.25 0.16 0.35 0.20 0.26 0.16 0.08	0.61 0.45 0.42 0.48 0.19 0.39 0.23 0.08	11 14 6 16 35 9 11 18
HORIZONTAL DISPLACE M = 5.3-5.7 class M = 6.4 class 1 M = 7.1-7.7 class M = 5.3-5.7 all M = 6.4 all San Fernando R1 San Fernando S1 San Fernando S2	1 9	1.81 1.48 2.34 1.60 1.91 2.72 2.07 2.09	-1.15 -0.55 -0.86 -1.03 -0.77 -1.52 -0.90 -0.76	0.36 0.30 0.22 0.34 0.28 0.25 0.25	0.59 0.53 0.56 0.46 0.27 0.38 0.37 0.18	11 14 6 16 35 9 11 18
VERTICAL ACCELERAT M = 5.0-5.7 class M = 6.0-6.4 class M = 7.1-7.7 class	1 14 1 15	-0.27 1.36 1.55	-0.77 -1.70 -1.58	0.29 0.20 0.21	0.36 0.32 0.39	19 16 8
VERTICAL VELOCITY: M = 5.3-5.7 class M = 6.4 class 1		1.62 1.86	-0.96 -0.80	0.30 0.18	0.48 0.32	11 14
VERTICAL DISPLACEM M = 5.3-5.7 class M = 6.4 class 1		1.22 1.15	-0.93 -0.53	0.29 0.14	0.47 0.25	11 14

APPENDIX B

STRONG MOTION DATA

Key for listing of strong motion data

Associated with each earthquake there is a six-digit number followed by a four-digit number. The first two digits of the six-digit number denote the year, the second two the month and the third two the day. The first two digits of the four-digit number represent the hour (Universal Time) and the second two the minute.

Abbreviations are explained below:

MAG - Earthquake Magnitude. Richter (1958) local magnitude if available, otherwise surface wave magnitude.

STA # - Station number as given by the U.S. Geological Survey (1976b).

STRUC - Code for associated structure. One if data were recorded at the base of a one- or two-story building, two if data were recorded at the base of a larger building or on a dam abutment.

DIST - Shortest distance to the surface of fault slippage.

AC - Accuracy code for distance. A if the uncertainty is less than 2 km, B if it is between 2 and 5 km, and C if it is between 5 and 25 km.

ACCEL - Peak acceleration as a fraction of the acceleration of gravity.

VEL - Peak velocity in cm/sec.

DISP - Peak displacement in cm.

 Duration in seconds, defined as the time interval between the first and last horizontal acceleration peaks equal to or greater than 0.05 g.

SRC - Code denoting source of strong motion data. List is given following the data.

GEO - Code for geologic conditions at recording site. S

for soil (greater than 4 to 5 meters in thickness) and R for rock.

- REF Code for source of information on stations. List of references follows station list.
- Denotes station selected from the special area in downtown Los Angeles as described in the text.

20224	1556	BEAR V	ALLEY.	CALIF	FORNIA				M	AG . 5.0				
	STATI					• HOR	IZONTAL		****	*****	VERTI	CAL **	••••	
3012		STRUC	DIST	AC	ACCEL		DISP			ACCEL	VEL	DISP	SRC	STATION LOCATION
	1028	1	31.0	A	0.030				В	0.010			8	HOLLISTER - CITY HALL
41128	2301	BEAR V	LLEY.	CALI	FORNIA			1	м	AG = 5.2				
						a uno	TONTAL			*****	VEDTT	CAL		
ROCK	STAT!	STRUC	DIST	AC			DISP				VEL	DISP	SRC	STATION LOCATION
	1032	1	18.0	A	0.011				E	0.013			E	SAGO CENTRAL - HARRIS RANCH
5011	STATI	ONS.	1		******	- HOR	IZONTAL		****	*****	VERTI	CAL	****	
3016		STRUC	DIST	AC			DISP			ACCEL		DISP		STATION LOCATION
	1377	1	8.9	A	0.120			1	G	0.050			G	SAN JUAN BAUTISTA (C126) - 24 POLK
	1028	1	10.8	A	0.170			1	G	0.070			G	HOLLISTER - CITY HALL
	1250	1	10.8		0.140			1	G	0.030			G	STONE CANYON EAST, CALIF.
	1202	1	37.0	^	0.030			1		0.030				STORE CANTON EAST, SALL,
50607	846	FERNDA	LE . CA	LIFOR	NIA			1	м	AG = 5.2				
ROCK	STATE	ONS:					IZONTAL							23 (2017) - 22 (201
	STA#	STRUC	DIST	AC	ACCEL	VEL	DISP	DUR	SRC	ACCEL	VEL	DISP	SRC	STATION LOCATION
	1249	1	32.0		0.220			1	I	0.030			1	CAPE MENDOCINO (CS) - PETROLIA SHELTER COVE, STA 2 (C41) - PWR PLT
	-	1									VEDTI			
SOIL	STAT!	STRUC	DIST	AC			DISP			ACCEL				STATION LOCATION
	1023	1	24.0		0.240			1	1	0.050			1	FERNDALE - OLD CITY HALL. BROWN ST
	1398	i	34.0		0.190				İ	0.030			1	
			1											
570322	1944	DALY C	ITY, C	ALIFO	RNIA					AG = 5.3				
ROCK	STAT						RIZONTAL			*****				STATION LOCATION
	STA#	STRUC	DIST	AC	ACCEL	VEL	DISP	DUR	SRC	ACCEL		DISP		
	1117	1	8.0	8	0.127	4.9	5.3	1.6	A	0.051	1.2	0.7	A	SAN FRANCISCO - GOLDEN GATE PARK
SOIL	STAT	IONS	4				RIZONTAL			*****				
	STA#	STRUC	DIST	AC	ACCEL	VEL	DISP	DUR	SRC	ACCEL	VEL	DISP	SRC	STATION LOCATION
	1080	2	12.0	В	0.103	5.0	141	1.4	A	0.050	2.3	0.6	A	SAN FRANCISCO - STATE BLDG
	1065	2	14.0	В	0.055	2.9	1.3	0.0	A	0.036	1.3	0.4	A	SAN FRANCISCO - ALEXANDER BLDG
	1078	5	14.0		0.048	5.0	1.4	0.0	A	0.034	1.5	0.9	A	SAN FRANCISCO - SOUTHERN PACIFIC BG OAKLAND - CITY HALL
1	1049	5	58.0		0.047	1.9	1.5	0.0	B	0.023	0.9	1.3	A	SAN JOSE - BANK OF AMERICA BLDG

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)			10	-												0
)	ROCK		NS1 STRUC	DIST	AC	ACCEL	++ HOR	IZONTAL DISP		SRC	ACCEL				STATION LOCATION	0
		290 111 116 278	1 1 1 2	15.0 18.0 19.0 32.0	8 8 8		9.6	2.2	2.8	AAAAA	0.076 0.093 0.094 0.018 0.016	3.2	1.4	A A A	WRIGHTWOOD - 6074 PARK DRIVE CEDAR SPRINGS - ALLEN RANCH DEVILS CANYON - FILTER PLANT SAN DIMAS - PUDDINGSTONE RESERVOIR ARCADIA - SANTA ANITA RESERVOIR	0
,		104 266 137 110	1 2 1	46.0 58.0 70.0 110.0	8	0.015 0.015 0.025				BAAA	0.010 0.006 0.011			B	PASADENA - CIT SEISMOLOGY LAB *LOS ANGELES - WATER & POWER CASTAIC - OLD RIDGE ROUTE	0
)	SOIL		NS: STRUC	DIST	AC	ACCEL		IZONTAL		SRC	ACCEL		DISP		STATION LOCATION	0
)		112 274	1 2	18.0	В	0.073	4.0	1.2	1.0	A	0.044 0.055 0.042	1.8	0.4 1.5 0.7	A	CEDAR SPRINGS - PUMP PLANT SAN BERNARDINO - HALL OF RECORDS COLTON - S. CAL. EDISON CO.	0
		113 129 264 267	2 2 2	29.0 34.0 57.0 60.0	8		1.5	1.8	0.0	8	0.009 0.015 0.017	0.7	0.5	B	LOMA LINDA - UNIV. MED. CENTER PASADENA - CIT MILLIKAN LIBRARY PASADENA - CIT JPL LAB	. 0
)		181 133 135	2 2	66.0 77.0 77.0	8	0.026				A	0.012 0.006 0.007			AAA	LOS ANGELES - 1640 SOUTH MARENGO *HOLLYWOOD STORAGE - BASEMENT *HOLLYWOOD STORAGE - P.E. LOT	0
)		125	i	95.0 113.0	В	0.010				B	0.006			B	LAKE HUGHES ARRAY 1 - FIRE STATION ANZA - ANZA POST OFFICE	C
)	660628	426	PARKFI	ELD. C	ALIFOR	RNIA			J	м	AG = 5.5					C
)	ROCK		ONS! STRUC	DIST	AC	ACCEL	VEL	DISP	DUR	SRC	ACCEL	VERTI	DISP	SRC	STATION LOCATION	C
)		1438 1083 110	1	16.1 63.6 204.0	A	0.411 0.018 0.004		5.5	3.7		0.165		1.4		CHOLAME-SHANDON! TEMBLOR SAN LUIS OBISPO - CITY REC. BLDG CASTAIC - OLD RIDGE ROUTE	0
)	SOIL		ONSI	DIST	AC			DISP			ACCEL		CAL .		STATION LOCATION	C
)	. 14	1013 1014	1	6.6	A	0.467	25.4	26.4	7.9		0.349	14.1	4.3	A	CHOLAME-SHANDON ARRAY NO. 2 CHOLAME-SHANDON ARRAY NO. 5 CHOLAME-SHANDON ARRAY NO. 8	0
)	_	1015 1016 1095	1 1	13.0 17.3 105.0	A	0.279 0.072 0.012	8.0	5.7 2.5	7.8 0.6 0.0	A	0.138 0.061 0.007	5.0 1.1	2.1	A	CHOLAME-SHANDON ARRAY NO. 12 TAFT - LINCOLN HS TUNNEL BUENA VISTA - GROUND STATION	
)		1011 1028 283 272	1 1	112.0 123.0 162.0 208.0	A	0.006 0.003 0.004 0.005			1	8 8	0.002			8	HOLLISTER - CITY HALL SANTA BARBARA - COURTHOUSE PORT HUENEME - NAVY LABORATORY	
)		133 135	2	261.0	A	0.001			100	B B					*HOLLYWOOD STORAGE - BASEMENT *HOLLYWOOD STORAGE - P.E. LOT PASADENA - CIT ATHENAEUM	
)		475	1	272.0		0.001										(
	670621 BOCK	1804 STATI		ANKS. A	LASKA		••• но	RIZONTA			AG = 5.6	VERT				
)	HOCK	-	STRUC	DIST	AC	ACCEL	VEL	DISP	DUR	SRC	ACCEL	VEL	DISP			C
0	-	2707	1	15.0		0.060				C	0.060			C	FAIRBANKS. ALASKA - UNIV OF ALASKA	

make men for any in the A-

In Fale (118)

	691002	456	SANTA	ROSA.	CALI	FORNIA				M	AG = 5.6					
	POCK	STATI	ONSI			*****		RIZONTAL		****	*****	VERTI	CAL	****		
	NOCK		STRUC	DIST	AC	ACCEL	. VEL	DISP	DUR	SRC	ACCEL	VEL	DISP	SRC	STATION LOCATION	
ij	7	1057	1	77.0		0.007				В	0.002			8	PLEASANT HILL - DIABLO VALLEY COL.	
8	1 1	1074	2	79.0		0.011				В	0.004			8		
	7							170NTAL			*****	VEDTT	CAL			
	SOIL	STATI	STRUC	DIST	AC	ACCEL		DISP					DISP		STATION LOCATION	
						1										
	2	1093	1 2	79.0		0.005			10	8	0.001			8	SAN PABLO - CONTRA COSTA COLLEGE SAN FRANCISCO - ALEXANDER BLDG	
		1071	2	79.0		0.015			-17	B	0.007			8	SAN FRANCISCO - BETHLEHEM PAC BLDG	
	20 1 40	1078	2	79.0		0.016				В	0.007			8	SAN FRANCISCO - SOUTHERN PACIFIC BG	
		1049	2	82.0		0.006			- 50	В	0.002			В	DAKLAND - CITT HALL	
		1001	1	110.0		0.018			- 10	8				8	APEEL ARRAY - STATION 2	
		1002	1 1	110.0	, ,	0.011					0.002				ar cee aman	
	691002		CANTA	BOCA .		FORMER					AG = 5.7					
	991005	619	SANTA	RUSA	CALI			els v								
	ROCK	STATI				******	+++ HOF	RIZONTAL	****	****	*****	VERTI	CAL *	****		
	3	STA#	STRUC	DIS	AC	ACCEL	VEL	DISP	DUR	SRC	ACCEL	VEL	DISP	SRC	STATION LOCATION	
	= .5	1057	1	77.) B	0.009				В	0.002			В		
		1074	2	79.) B	0.012				В	0.004			В	SAN FRANCISCO - 390 MAIN	
	2011	CTATI	ONE	1		*****		RIZONTAL			*****	VEDTT	CAL .			
	301L	STATI	STRUC	DIS	TAC	ACCEL	VEL	DISP	DUR	SRC	ACCEL				STATION LOCATION	
	F .															
	1. 1	1093	1		0 8	0.003				В	0.003			В	SAN PABLO - CONTRA COSTA COLLEGE SAN FRANCISCO - ALEXANDER BLDG	
	e 1111	1065		79.	0 8	0.012				B	0.003			B	SAN FRANCISCO - BETHLEHEM PAC BLDG	
		1078	2	79.		0.020			- 7	A	0 000			В	SAN FRANCISCO - SOUTHERN PACIFIC BG	
		1049	2	82.	0 B	0.013			1	В	0.004			В	OAKLAND - CITY HALL	
		1001		109.	0 B	0.029				В				8	APEEL ARRAY - STATION 1	
		1002	1	110.	0 B	0.021			1	В	0.009			В	APEEL ARRAY - STATION 2	
	III A TOTAL		1	1					11	45						
	750801	2020	OROVI	LLE, C	ALIFO	RNIA				м	AG = 5.7					
	ROCK	STAT		1		*****	*** HO	RIZONTAL	****	****	*****	VERTI	CAL .	****	The second secon	
		STA#	STRUC	DIS	TAC	ACCEL	VEL	DISP	DUR	SRC	ACCEL	VEL	DISP	SRC	STATION LOCATION	
		1051	1	8.	0 A	0.110	5.0	1.6	0.0	н	0,120	5.3	2.7	н	OROVILLE SEISMOGRAPH STATION	
	3	1293			0 A	0.040				н	0.030			н	PARADISE (C58) - KEWG TRNSMTR BLDG	
	SOTI	STAT	IONS					RIZONTAL			*****	VERTI	CAL .			
	3016		STRUC		T AC	ACCEL	VEL	DISP	DUR	SRC	ACCEL					
	1 4 4														MARYSVILLE (C56) - CDOT MAINT BLDG	
		1291			0 A	0.070				H	0.040				CHICO (C57) - 2334 FAIR STREET	
		1272		31.		V. VOV				110						
	*****	1440	00111	Milair							AG = 6.0					
	730221	1445	POINT	MUGU +	CALI	FORNIA				-	AU = 0.0					
	ROCK	STAT	IONSI	1		****	HO	RIZONTAL	****	****	*****	VERT	ICAL .	****		
			****			ACCEL	VE	DIED	DUD	SPC	ACCEL	VEI	DISD	SRC	STATION LOCATION	
		STA#	STRUC	015	I AC	ACCEL	AFF	DISF	DUK	SAC	ACCEL	ACL	UISF		SINITON LOCALION	

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																0
-11		655	1	53.0	В	0.031				ε	0.014				JENSEN FILTER PLT - 13100 BALBOA. LA	0
	SOIL	STATI	ONS! STRUC	DIST	10	ACCEL	HOR	IZONTAL DISP	****	**** SPC	ACCEL		CAL		STATION LOCATION	
•	1113	272	1	24.0		0.130	VEL	DISF	DON	0	0.040	,,,,	0131	D	PORT HUENEME - NAVY LABORATORY	0
•		610 657 118	2 2	51.0 51.0 53.0	8	0.043 0.036 0.042				E	0.016 0.012 0.016			EEE	LOS ANGELES - 18321 VENTURA SANTA MONICA - 201 OCEAN LOS ANGELES - 16661 VENTURA	0
9		497 512 259	2 2	53.0 54.0 55.0	8 8 8	0.060 0.036 0.032				E	0.010 0.016 0.013			DEE	LOS ANGELES - 16633 VENTURA LOS ANGELES - 16255 VENTURA LOS ANGELES - 16055 VENTURA	0
0		461	2	55.0	В	0.040				Ε	0.023			E	LOS ANGELES - 15910 VENTURA	0
	721223	629	MANAGUA	, NIC	RAGUA					M	AG = 6.2					0
	SOIL	STATI STA#	ONS:	DIST	AC	ACCEL	** HOR	DISP	DUR	SRC	ACCEL	VERTI	CAL **	SRC	STATION LOCATION	0
•		3501	1	5.0	A	0.390			1	D	0,330			D	MANAGUA+ NIC ESSO REFINERY	
0	400519	436	IMPERIA	AL VALI	FY. C	AL TEORN	tá		1	м	AG . 6.4				4	0
0		STATI						TONTAL			*****		CAL			0
	3014		STRUC	DIST	AC	ACCEL	VEL	DISP	DUR	SRC	ACCEL	VEL	DISP	SRC	STATION LOCATION	0
•	- 0.4	117	1	12.0	8	0.359	36.9	19.8	29.3	A	0.278	10.8	5.6	A	EL CENTRO - IRRIGATION SUBSTA.	0
•	680409	228	BORREGO	O MTN.	, CAL						AG = 6.4				A	0
•	ROCK		ONS:	DIST	AC	ACCEL	VEL	DISP	DUR	SRC	ACCEL	VERT	DISP	SRC	STATION LOCATION	0
•		270 280	1	105.0 122.0 141.0	A	0.018 0.048 0.011	4.2	2.9	0.0	F.	0.006	3.7	1.7	FA	PERRIS - RESERVOIR SAN ONOFRE - SCE NUCLEAR PLANT DEVILS CANYON - FILTER PLANT	0
•		116 278 104	2 2	168.0	A	0.017				F	0.004			F	SAN DIMAS - PUDDINGSTONE RESERVOIR ARCADIA - SANTA ANITA RESERVOIR PASADENA - CIT SEISMOLOGY LAB	0
0	1	266 136 190	5	200.0	A	0.007 0.012 0.007	3.1	2.3	0.0	Å	0.002	1.2	1.0	A.F.	*LOS ANGELES - SUBWAY TERMINAL LOS ANGELES - 2011 ZONAL SAN FERNANDO - PACOIMA DAM	0
0	11	279 121 110	2 2	229.0 249.0 256.0	A	0.009 0.003 0.008				F	0.006 0.001 0.003			F	FAIRMONT STATION - RESERVOIR CASTAIC - OLD RIDGE ROUTE	0
0	SOIL		ONS:	DIST	AC	ACCEL		DISP		SRC	ACCEL		CAL .		STATION LOCATION	0
0	11	117	1	45.0 105.0	A	0.142		12.2	3.1		0.036	3.4	3.9	A	EL CENTRO - IRRIGATION SUBSTA. SAN DIEGO - LIGHT & POWER	0
0		113 274	1 2	130.0	A	0.031	3,5		0.0		0.022	1.8	1.1	F	COLTON - S. CAL. EDISON CO. SAN BERNARDINO - HALL OF RECORDS CEDAR SPRINGS - PUMP PLANT	0
		281	5	147.0	A	0.006			0.0		0.006		1.9	A	SANTA ANA - ORANGE CO. ENG. BLDG LONG BEACH - TERMINAL ISLAND	
0		130 131 288	2	187.0 187.0 196.0	A	0.010 0.005 0.019	3.2		0.0	F	0.006 0.003 0.008	2.4	1.8	F	LONG BEACH - TERMINAL ISLAND LONG BEACH - UTILITIES BLDG. VERNON - CENTRAL MFG. TERMINAL	0
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T10209 1400 SAN FERNANDO, CALIFORNIA ***********************************											-	4.400			4		
### T10209 1400 SAN FERNANDO, CALIFORNIA #### ROCK STATIONS: STAW STRUC				-								0.001					
ROCK STATIONS: STAM STRUC DIST AC ACCEL VEL DISP DUR SRC ACCEL VEL DISP SRC STATION DIST AC ACCEL VEL DISP DUR SRC ACCEL VEL DISP SRC STATION DATA 220 2 16.9 A 0.881 15.0 5.4 6.7 A 0.885 5.0 2.4 A LOS ANGELES - 3838 LANKERSHIM 266 1 18.4 A 0.204 11.6 5.0 6.7 A 0.993 5.9 2.3 A PASADENA - CIT SISMOLOGY LAB 141 1 19.4 A 0.888 20.5 7.3 9.6 A 0.138 7.4 3.4 A LOS ANGELES - GRIFFITH OBSERVATORY 128 1 21.0 A 0.374 14.6 8.9 14.5 A 0.164 4.1 3.3 A LAKE HUGHES ARRA' 12 - CWR SITE 127 1 24.0 A 0.204 8.6 1.7 5.7 A 0.170 7.1 1.6 A LAKE HUGHES ARRA' 12 - CWR SITE 127 1 24.0 A 0.214 4.8 2.4 4.6 A 0.888 3.0 2.2 A LAKE HUGHES ARRA' 12 - CWR SITE 110 1 2 26.0 A 0.233 1.8 9.9 10.9 A 0.070 4.5 2.5 A CACAIT - SLOTH CARRA' 4 - CWR SITE 110 1 2 26.0 A 0.233 1.8 9.9 10.9 A 0.070 4.5 2.5 A CACAIT - SLOTH CARRA' 4 - CWR SITE 110 1 2 2 2.4 A 0.233 1.8 9.9 10.9 A 0.070 4.5 2.5 A CACAIT - SLOTH CARRA' 4 - POWER 111 2 2 30.0 A 0.188 23.4 13.7 0.4 A 0.078 10.3 1.8 A LOS ANGELES - WATER & POWER 127 2 3 0.0 A 0.103 8.4 1.7 1.8 A 0.043 3.4 1.7 A FAIRMONT STATION - RESERVOIR 278 2 4.7.0 A 0.078 4.6 2.1 1.7 A 0.039 2.3 1.8 A LOS ANGELES - WATER & POWER 1096 1 64.0 A 0.025 1.4 0.0.0 A 0.00		1095	•	1	359.0	A	0.002				F					TAFT . LINCOLN HS TUNNEL	
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125 1 27.0 A 0.152 17.9 3.4 13.1 A 0.102 11.7 2.8 A LAKE HUGHES ARRAY 1 - FIRE STATION																	
City u Attac 1141 Att 1041 U Attac 1141 Eto u church dimini e i the anniam											A				A	LAKE HUGHES ARRAY 1 - FIRE STATION	
		123			21.00	-	01132	11.03	3.7			41102		2.00		THE HOUSE WHILE STATES	

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	262	1	32.0		0.150	14.2	3.8	14.5	A .	0.105	7.8	2.4		PALMDALE - FIRE STATION
	288 244	5	33.0 36.0	A	0.111	17.5	14.8	9.1	A	0.047	6.7	3.9	Ā	VERNON - CENTRAL MFG. TERMINAL LOS ANGELES - 8639 LINCOLN
	247 229 269	2	37.0 37.0 41.0	A	0.069	5.4		4.6	A	0.025 0.028 0.056 0.041	5.7	3.5 3.6 1.7 1.2	A	LOS ANGELES - 9841 AIRPORT BLVD LOS ANGELES - 5250 CENTURY BLVD PEARBLOSSOM - PUMPING PLANT OSO PUMPING PLANT
	1052 411 131 132	1 2	54.0 58.0 58.0		0.112 0.043 0.028 0.038	8.5 5.0 9.6 9.5	2.3 3.4 7.3 8.0	0.0	A	0.020 0.015 0.027	3.8 2.2 6.1 4.9	1.3	A	PALOS VERDES - 2516 VIA TEJON LONG BEACH - UTILITIES BLDG. LONG BEACH - STATE COLLEGE
10	476 130 272	1		A	0.040 0.030 0.027	5.8 10.4 7.3	2.7 8.7 4.9	0.0	A	0.017 0.016 0.011	2.3 4.2 3.2	1.9	A	FULLERTON - 2600 NUTWOOD AVE. LONG BEACH - TERMINAL ISLAND PORT HUENEME - NAVY LABORATORY
	472 281 114	5 5	66.0 70.0 78.0		0.033 0.029 0.036	8.5 8.0 7.0	6.5 5.7 6.9	0.0	A	0.020 0.020 0.010	3.9 2.4 3.5	2.5 1.7 2.3	A	ORANGE - 400 W. CHAPMAN SANTA ANA - ORANGE CO. ENG. BLDG COSTA MESA - 666 W. NINETEENTH
	1102 112 113	1 1	91.0	A	0.034 0.030 0.039	2.5	1.3	0.0	A	0.015 0.013 0.026	1.5	1.3	A	WHEELER RIDGE - GROUND STATION CEDAR SPRINGS - PUMP PLANT COLTON - S. CAL. EDISON CO.
1	274 465 123 103	1	93.0 104.0 134.0 168.0		0.047 0.044 0.044 0.037	3.5 4.6 2.9 2.6	1.7	0.0	A	0.019 0.022 0.027 0.015	1.5 3.4 2.3 1.4	1.6 1.3	A	SAN BERNARDINO - HALL OF RECORDS SAN JUAN CAPISTRANO - CITY HALL HEMET - FIRE STATION ANZA - ANZA POST OFFICE
			2			2.00		11				•••		
490413	1955	PUGET	SOUND,	WASHI			HAT I	in.		AG = 7.1				
SOIL	STATI STA#	STRUC	DIST	AC	ACCEL		DISP		SRC	ACCEL		DISP		STATION LOCATION
	2101 2170	1	48.0 69.0				10.4		A	0.111	7.0	2.3		OLYMPIA - HIGHWAY TEST LAB SEATTLE ARMY BASE - 4735 E MARGINAL
590818	637	HEBGEN	LAKE.	MONTA	INA			1	м	AG = 7.1				*
ROCK	STATI STA#	ONS:	DIST	AC	ACCEL		DISP		SRC	ACCEL		DISP		STATION LOCATION
	2201 2202 2204	1 2 1	175.0 208.0 454.0	C	0.043 0.013 0.001			+	8 8 8	0.021 0.008 0.001			B B	BUTTE, MONT SCHOOL OF MINES HELENA, MONT CARROL COLLEGE HUNGRY HORSE - DOWNSTREAM STATION
SOIL	STATI	ONS: STRUC	DIST	AC	ACCEL		DISP		SRC	ACCEL		CAL		STATION LOCATION
	2205	2	95.0	C	0.055				8	0.026			В	BOZEMAN, MONT STATE COLLEGE
720730	2145	SITKA	ALASK	Α.		•			м	AG = 7.6				
ROCK	STATI	ONS: STRUC	DIST	AC	ACCEL		DISP		SRC	ACCEL				STATION LOCATION
	2714 2708	1	45.0 145.0		0.110				B B	0.050			8	SITKA: ALASKA - MAGNETIC OBS. JUNEAU: AUKE BAY - BUR OF COMM FISH
SOIL		ONS:	DIST	AC	ACCEL	* HOF	DISP	DUR	SRC	ACCEL	VERTI	CAL	SRC	STATION LOCATION
	2715	1	300.0	В	0.010				В					YAKUTAT, ALASKA - ATRPORT PUMP HOUSE

promote the following of the first of the following of the following of the following of the first of the fir

0721	1152	KERN C	OUNTY	CALI	FORNIA				M	AG = 7.7				
-	STATI	ouc.			*****	HOR	TONTAL	****	****	*****	VERTI	CAL	****	
RUCK		STRUC	DIST	AC	ACCEL		DISP		SRC	ACCEL	VEL	DISP	SRC	STATION LOCATION
	136 1083	2	115.0 148.0		0.032				8	0.008			8	+LOS ANGELES - SUBWAY TERMINAL SAN LUIS OBISPO - CITY REC. BLDG
***	STATI	ONE .			*****	** HOR	IZONTAL		****	*****	VERTI	CAL	****	
3011		STRUC	DIST	AC	ACCEL		DISP		SRC	ACCEL	VEL	DISP	SRC	STATION LOCATION
	1095 283 133 135 475 288 131 113 1008 277 1028 2001 1081 117 1049 1078	1 1 2 1 1 2 2 2 1 1 1 2 2 1 1 2 2 1 1 2 1 1 2 1 2 1 2 1 2 1 2	42.0 85.0 107.0 107.0 122.0 145.0 156.0 224.0 293.0 359.0 366.0 370.0 425.0	8 8 8 8 8 8 8 8 8 8	0.196 0.135 0.058 0.062 0.054 0.014 0.018 0.005 0.010 0.004 0.004 0.004 0.004	17.7 19.3 9.4 8.9 9.1	9.1 5.8 5.9 6.4 2.9	19.6 13.8 0.1 0.1 0.1	A A A A B B B B B B B B B B B B B B B B	0.123 0.051 0.024 0.022 0.033 0.012 0.006 0.012 0.006 0.001	6.7 5.0 4.2 3.1 4.5	5.0 2.1 2.2 3.4 3.0	A A A A B B B B B B B B B B B B B B B B	TAFT - LINCOLN HS TUNNEL SANTA BARBARA - COURTHOUSE CHOLLYWOOD STORAGE - BASEMENT CHOLLYWOOD STORAGE - P.E. LOT PASADENA - CIT ATHENAEUM VERNON - CENTRAL MFG. TERMINAL LONG BEACH - UTILITIES RLDG. COLTON - S. CAL. EDISON CO. BISHOP - LA WATER DEPT GARAGE SAN DIEGO - LIGHT & POWER HOLLISTER - CITY HALL HAWTHORNE - US NAVY AMMO. DEPOT SAN JOSE - BANK OF AMERICA BLDG EL CENTRO - IRRIGATION SUBSTA. OAKLAND - CITY HALL SAN FRANCISCO - SOUTHERN PACIFIC BG
OURCES	OF ST	RONG MO	TION D	ATA				-						3.
		ERENCE	3											- No.

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		CTOUG		LOCATION	STRUCTURE	REF	GEOLOGY	RI
	STAN	STRUC	GEO	LOCATION	STRUCTURE	HEF	GEOLOGY	-
ł	103	1	S	ANZA - ANZA POST OFFICE	1 STORY BLDG	1	ALLUVIUM	-
	104	2	R	ARCADIA - SANTA ANITA RESERVOIR	ABUTMENT, C DAM	1	GRANITE/DIORITE	
	110	1	R	CASTAIC - OLD RIDGE ROUTE	INST SHELTER	1	SANDSTONE	
	111	i	R	CEDAR SPRINGS - ALLEN RANCH	1 STORY BLDG	1	GRANITIC	
	112	î	S	CEDAR SPRINGS - PUMP PLANT	1 STORY BLDG	i	SHALLOW ALLUVIUM	
		i	Š	COLTON - S. CAL. EDISON CO.	1 STORY BLDG	i	DEEP ALLUVIUM	
	113	ż	5	COSTA MESA - 666 W. NINETEENTH	18 STORY BLOG	i	ALLUVIUM	
	116	1	R	DEVILS CANYON - FILTER PLANT	1 STORY BLDG	i	LS/GNEISS	
	117	i	5	EL CENTRO - IRRIGATION SUBSTA.	2 STORY BLDG	i	>300 M ALLUVIUM	
			5	LOS ANGELES - 16661 VENTURA	8 STORY RC BLDG	i	BM ALLUV/SHALE	
	118	2	R	FAIRMONT STATION - RESERVOIR	ABUTHENT . E DAM	î	GRANITE	
	121	2			3 STORY BLDG		>8 M ALLUVIUM	
	155	5	S	GLENDALE - 633 E. BROADWAY		1		
	123	1	S	HEMET - FIRE STATION	1 STORY BLDG	1	ALLUVIUM	
	125	1	5	LAKE HUGHES ARRAY 1 - FIRE STATION	1 STORY BLDG	1	300 M ALLUVIUM	
	126	1	R	LAKE HUGHES ARRAY 4 - CWR SITE	INST SHELTER	1	WEATHERED GRANIT	
	127	1	R	LAKE HUGHES ARRAY 9 - CWR SITE	1 STORY BLDG	1	GNETSS	
	128	1	R	LAKE HUGHES ARRAY 12 - CWR SITE	1 STORY BLDG	1	THIN ALLUVIUM	
	129	5	S	LOMA LINDA - UNIV. MED. CENTER	10 STORY BLDG	3	APP 250 M ALLUV	
	130	1	S	LONG BEACH - TERMINAL ISLAND	1 STORY BLDG	1	DEEP ALLUVIUM	
	131	2	S	LONG BEACH - UTILITIES BLDG.	4 STORY BLDG	1	DEEP ALLUVIUM	
	132	2	S	LONG BEACH - STATE COLLEGE	9 STORY BLOG	1	>15 M ALLUVIUM	
	133	2		HOLLYWOOD STORAGE - BASEMENT	14 STORY RC	1	130 M ALLUVIUM	
	135	1		HOLLYWOOD STORAGE - P.E. LOT	INST SHELTER	1	130 M ALLUVIUM	
	136	5		LOS ANGELES - SUBWAY TERMINAL	12 STORY BLDG	3	120 M SHALE	
	137	5	R .	LUS ANGELES - WATER & POWER	15 STORY STEEL	1	MIOCENE SILTSTNE	
	141	1	R	LOS ANGELES - GRIFFITH OBSERVATORY	INST SHELTER	1	GRANITE	
	181	2	S	LOS ANGELES - 1640 SOUTH MARENGO	7 STORY RC	1	>16 M ALLUVIUM	
	190	5	R	LOS ANGELES - 2011 ZONAL	9 STORY RC	1	ALLUVIUM 0-10 M	
	220	5	R	LOS ANGELES - 3838 LANKERSHIM	20 STORY RC	1	SH & SS	1
	229	2	S	LOS ANGELES - 5250 CENTURY BLVD	7 STORY STEEL	1	>16 M ALLUVIUM	
	241	2	S	LOS ANGELES - 8244 ORION	7 STORY RC	1	>13 M ALLUVIUM	
	244	2	5	LOS ANGELES - 8639 LINCOLN	12 STORY RC	1	>18 M ALLUVIUM	
	247	5	S	LOS ANGELES - 9841 AIRPORT BLVD	14 STORY RC	1	>23 M ALLUVIUM	
	253	2	S	LOS ANGELES - 14724 VENTURA	12 STORY RC	1	>24 M ALLUVIUM	
	259	2	5	LOS ANGELES - 16055 VENTURA	12 STORY BLDG	1	12M ALLUV/SHALE	
	262	1	S	PALMDALE - FIRE STATION	1 STORY BLDG	1	ALLUVIUM	
	264	2	5	PASADENA - CIT MILLIKAN LIBRARY	9 STORY RC	1	APP 300 M ALLUV	
	266	1	R	PASADENA - CIT SEISMOLOGY LAB	2 STORY BLDG	1	GRANITE	
	267	2	S	PASADENA - CIT JPL LAB	9 STORY STEEL	1	SANDY GRAVEL	
	269	ī	S	PEARBLOSSOM - PUMPING PLANT	INST SHELTER	1	130 M ALLUVIUM	
	270	i	R	PERRIS - RESERVOIR	INST SHELTER	1	ALLUV VEN/GRANIT	
	272	1	S	PORT HUENEME - NAVY LABORATORY	1 STORY WAREHSE	1	>300 M ALLUVIUM	
	274	ż	Š	SAN BERNARDING - HALL OF RECORDS	6 STORY BLDG	1	>35 M ALLUVIUM	
	277	5	S	SAN DIEGO - LIGHT & POWER	4 STORY BLOG	1	DEEP ALLUVIUM	
	278	2	R	SAN DIMAS - PUDDINGSTONE RESERVOIR	ABUTMENT-EARTH	î	VOL CLASTICS-SH	
	279	5	R	SAN FERNANDO - PACOIMA DAM	ABUTMENT-CONCRET	i	JOINTED GNEISS	
			R	SAN ONOFRE - SCE NUCLEAR PLANT	1 STORY WAREHSE	i	SOFT SANDSTONE	
	280	1			3 STORY BLDG	i	ALLUVIUM	
	281	5	5	SANTA ANA - ORANGE CO. ENG. BLDG	1 STORY BLDG	1	4 M ALLUV/SILTST	
	282	1	R	GOLETA - UC FLUID MECHANICS LAB	2 STORY BLDG	i	>10 M ALLUVIUM	
	283	1	_	SANTA BARBARA - COURTHOUSE			DEEP ALLUVIUM	
	288	5	S	VERNON - CENTRAL MFG. TERMINAL	6 STORY BLDG	1	ALLUV VEN/IGN	
	290	1	R	WRIGHTWOOD - 6074 PARK DRIVE	2 STORY BLDG	1		
	319	2	S •		4 STORY BLDG	3	21 M ALLUVIUM	
į.	411	1	S	PALOS VERDES - 2516 VIA TEJON	2 STORY BLDG		SHALLOW SANDS/SH	
	458	2	S	LOS ANGELES - 15107 VAN OWEN	7 STORY RC		>23 M ALLUVIUM	
	461	2	S	LOS ANGELES - 15910 VENTURA	18 STORY STEEL	1	>12 M ALLUVIUM	

City Common		Service Pr	T SOUTH THE						
0	7.12				1 57004 0106	1	ALLUVIUM	2	
	465	1	S	SAN JUAN CAPISTRANO - CITY HALL	1 STORY BLDG	1	>12 M ALLUVIUM	ī	
	466	2	S	LOS ANGELES - 15250 VENTURA	12 STORY RC 19 STORY BLDG	i	>100 M ALL/SHALE	2	
-	472	5	S	ORANGE - 400 W. CHAPMAN	2 STORY RC	3	APPROX 200 M ALL	3	
	475	1	5	PASADENA - CIT ATHENAEUM	10 STORY RC	1	>20 M ALLUVIUM	1	
	476	5	5	FULLERTON - 2600 NUTWOOD AVE.	12 STORY STEEL	i	APPROX 100 M ALL	ż	
	482	2	S	ALHAMBRA - 900 SOUTH FREEMONT	14 STORY BLDG	1	ALLUVIUM	1	
1 17	497	5	5	LOS ANGELES - 16633 VENTURA	12 STORY BLDG	i	20M ALLUV/SHALE	1	
	512	2	5	LOS ANGELES - 16255 VENTURA	10 STORY BLOG	i	>5M ALLUVIUM	1	
-	610	2	S	LOS ANGELES - 18321 VENTURA	2 STORY BLOG	i	ROCK	7	
	655	1	R	JENSEN FILTER PLT - 13100 BALBOA+ LA	18 STORY BLDG	i	SOIL	7	
	657	2	5	SANTA MONICA - 201 OCEAN		i	210M ALLUVIUM	1	
	1001	1	S	APEEL ARRAY - STATION 1	INST SHELTER	1	8M MUD/85M ALLUV	1	
	1002	1	S	APEEL ARRAY - STATION 2	INST SHELTER AUDITORIUM	i	>250 M ALLUVIUM	i	
•	1004	1	5	BAKERSFIELD - HARVEY AUDITORIUM	1 STORY BLDG	3	200 M ALLUVIUM	3	
	1008	1	S	BISHOP - LA WATER DEPT GARAGE		1	ALLUVIUM	2	
	1011	1	S	BUENA VISTA - GROUND STATION	INST SHELTER	1	45 M ALLUV/SS	1	
	1013	1	5	CHOLAME-SHANDON ARRAY NO. 2	INST SHELTER	;	ALLUVIUM	1	
•	1014	1	S	CHOLAME-SHANDON ARRAY NO. 5		1	THIN ALLUVIUM/SS	1	
	1015	1	S	CHOLAME-SHANDON ARRAY NO. 8	1 STORY BLDG	•	30 M TERRACE/SS	1	
	1016	1	S	CHOLAME-SHANDON ARRAY NO. 12	INST SHELTER	i	ALLUVIUM	i	
	1023	1	S	FERNDALE - OLD CITY HALL, BROWN ST	2 STORY BLDG	i	5 M ALLUV/GNEISS	2	
	1027	1	R	EDMONSTON - GROUND STATION	INST SHELTER	3	13 M ALLUVIUM	3	
	1028	1	5	HOLLISTER - CITY HALL	1 STORY BLDG	1		6	
	1032	1	R	SAGO CENTRAL - HARRIS RANCH	INST SHELTER	3	76 M MUD-ALLUVIU	3	
7.3	1049	5	S	OAKLAND - CITY HALL	15 STORY BLDG	1	METAVOLCANICS	1	
	1051	1	R	OROVILLE SEISMOGRAPH STATION	1 STORY BLDG	1	ALLUVIUM	,	
-	1052	1	5	OSO PUMPING PLANT	INST SHELTER	3	2 M ALUV/SS	3	
	1057	1	R	PLEASANT HILL - DIABLO VALLEY COL.	2 STORY BLOG	3	46 M ALLUVIUM	3	
•	1065	5	S	SAN FRANCISCO - ALEXANDER BLDG	15 STORY BLDG		70M ALLUVIUM	1	
-	1071	5	5	SAN FRANCISCO - BETHLEHEM PAC BLDG	14 STORY BLDG	1	SHALE/SS	i	
	1074	2	R	SAN FRANCISCO - 390 MAIN	7 STORY BLDG	3	90 M FILL-ALLUV	3	
	1078	2	5	SAN FRANCISCO - SOUTHERN PACIFIC BG	12 STORY BLDG	3	61 M ALLUVIUM	3	
	1080	5	S	SAN FRANCISCO - STATE BLDG	7 STORY BLDG	3	APPROX 750 M ALL	3	
	1081	5	S	SAN JOSE - BANK OF AMERICA BLDG	13 STORY BLDG	i	2 M LOAM/FRAN SH	2	
	1083	1	R	SAN LUIS OBISPO - CITY REC. BLDG	2 STORY BLDG 2 STORY BLDG	3	6 M FILL-ALLUV	3	
	1093	1	5	SAN PABLO - CONTRA COSTA COLLEGE	1 STORY SCH BLDG	1	ALLUVIUM	1	
	1095	1	S	TAFT - LINCOLN HS TUNNEL	1 STORY BLOG	1	GRANITE	1	
•	1096	1	R	FORT TEJON - CWR SITE	INST SHELTER	•	APPROX 100 M ALL	2	
- 13	1102	1	5	WHEELER RIDGE - GROUND STATION	INST SHELTER	1	FRAN CHERT-SHALE	3	
	1117	1	R	SAN FRANCISCO - GOLDEN GATE PARK	1 STORY BLOG	1	SOIL	8	
	1202	1	5	STONE CANYON EAST CALIF.	INST SHELTER	i	CRETACEOUS ROCK	1	
-	1249	1	R	CAPE MENDOCINO (C5) - PETROLIA	1 STORY BLOG	i	TERRACE DEPOSITS	1	
	1250	1	R	GILROY (C6) - GEOL BLDG, GAL COL SHELTER COVE, STA 2 (C41) - PWR PLT	INST SHELTER	1	FRANCISCAN ROCK	1	
	1278	1	S	MARYSVILLE (C56) - CDOT MAINT BLDG	1 STORY BLDG	1	100M ALLUVIUM	1	
-	1291	1	S	CHICO (C57) - 2334 FAIR STREET	ISTORY BLDG	i	90M ALLUVIUM	1	
	1292	1	R	PARADISE (C58) - KEWG TRNSMTR BLDG	1 STORY BLDG	1	VOLCANIC ROCK	1	
	1293	1	S	SAN JUAN BAUTISTA (C126) - 24 POLK	1 STORY BLDG	i	SOIL	6	
-	1377	i	5	PETROLIA (C156) - GENERAL STORE	INST SHELTER	i	ALLUVIUM	1	
	1438	i	R	CHOLAME-SHANDON: TEMBLOR	INST SHELTER	1	ROCK	9	
	2001	î	S	HAWTHORNE - US NAVY AMMO. DEPOT	1 STORY BLOG	1	ALLUVIUM	1	
	2005	2	S	MOJAVE GENERATING PLANT	LRG POWER PLANT	3	APPROX 70 M ALLU	3	
		1	S	OLYMPIA - HIGHWAY TEST LAB	INST SHELTER	1	ALLUVIUM	5	
	2101	i	S	SEATTLE ARMY BASE - 4735 E MARGINAL	1 STORY BLDG	1	ALLUVIUM	5	
-	2201	i	R	BUTTE MONT - SCHOOL OF MINES	2 STORY BLDG	3	GRANITIC INTRUS	3	
	2202	2	R	HELENA, MONT CARROL COLLEGE	5 STORY BLDG	3	GRANITICS	3	
	-		R	HUNGRY HORSE - DOWNSTREAM STATION	INST SHELTER	3	LIMESTONE	3	
	2204	1 2	S	BOZEMAN MONT - STATE COLLEGE	3 STORY BLDG	3	APPROX 170 M ALL	3	
	2205	1	R	FAIRBANKS ALASKA - UNIV OF ALASKA	INST SHELTER	3	SCHIST	3	
	2707		R	JUNEAU - AUKE BAY - BUR OF COMM FISH	1 STORY BLOG	1	SLATE	1	
100	2708	1	R	SITKA ALASKA - MAGNETIC OBS.	INST SHELTER	i	GRAYWACKE	1	
	2714	1	-	STIME HEADING - HAUNELIE UDS.			200		

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YAKUTAT. ALASKA - AIRPORT PUMP HOUSE 1 STORY BLDG GLACIAL OUTWASH 2715 MANAGUA. NIC. - ESSO REFINERY 1 STORY BLDG ALLUVIUM 3501 REFERENCE LISTING CODE REFERENCE U.S. GEOLOGICAL SURVEY, 1976, STRONG-MOTION ACCELEROGRAPH STATION LIST -1975: U.S. GEOLOGICAL SURVEY OPEN-FILE REPORT NO. 76-79. MALEY, R.P., AND CLOUD, W.K., 1973, STRONG-MOTION ACCELEROGRAPH RECORDS. IN: SAN FERNANDO, CALIFORNIA, EARTHQUAKE OF FEBRUARY 9, 1971, 0. 3: U.S. DEPARTMENT OF COMMERCE. NATIONAL OCEANIC AND ATMOSPHERIC ADMINISTRATION. MALEY. R.P., 1975. WRITTEN COMMUNICATION. VALERA. J.E. . 1973, SOIL CONDITIONS AND LOCAL SOIL EFFECTS DURING THE MANAGUA EARTHOUAKE OF DECEMBER 23, 1972, 'IN' MANAGUA, NICARAGUA EARTHQUAKE OF DECEMBER 23, 1972, VOLUME I: EARTHQUAKE ENGINEERING RESEARCH INSTITUTE, P.232-264. TRIFUNAC. M.D. AND BRADY. A.G. 1975. ON THE CORRELATION OF SEISMIC INTENSITY SCALES WITH THE PEAKS OF RECORDED STRONG GROUND MOTION: SEISMOL. SOC. AMERICA BULL., V. 65, P. 139-162. JENNINGS. C.W. AND STRAND, R.G. , 1959. GEOLOGIC MAP OF CALIFORNIA. SANTA CRUZ SHEET, SCALE 1:250,000, CALIFORNIA DIVISION OF MINES AND GEOLOGY. JENNINGS, C.W. AND STRAND, R.G., 1969, GEOLOGIC MAP OF CALIFORNIA, LOS ANGELES SHEET, SCALE 1:250.000, CALIFORNIA DIVISION OF MINES AND GEOLOGY. MALEY. R.P. . 1977, ORAL COMMUNICATION. CLOUD. W.K., AND PEREZ, V., 1967, ACCELEROGRAMS - PARKFIELD EARTHQUAKE: SEISMOL. SOC. AMERICA BULL. V.57. P.1179-1192.

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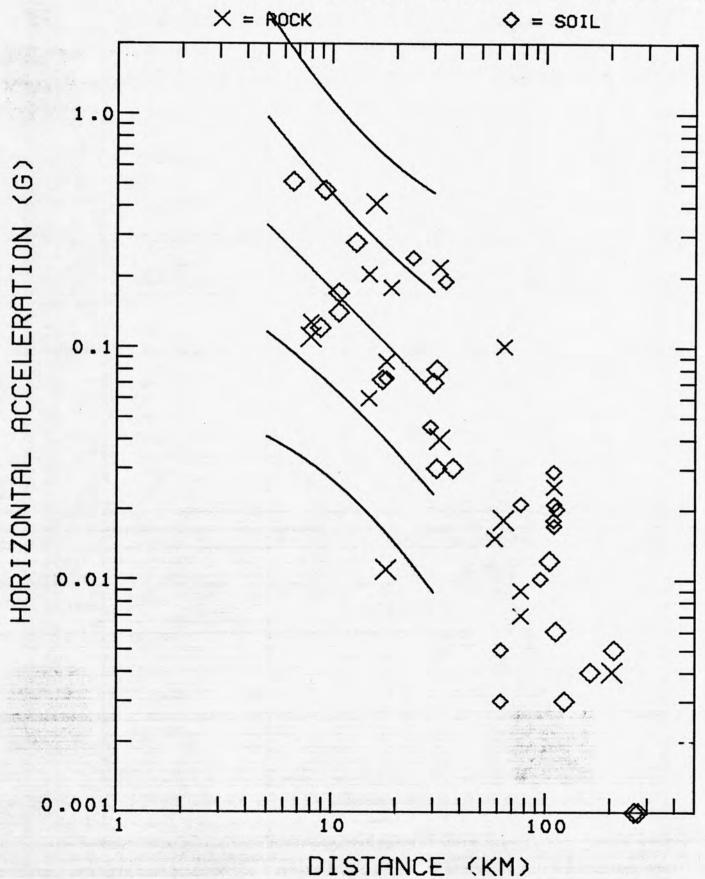
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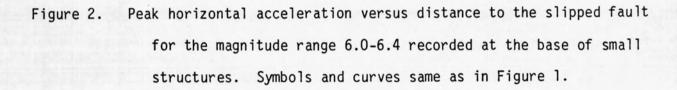
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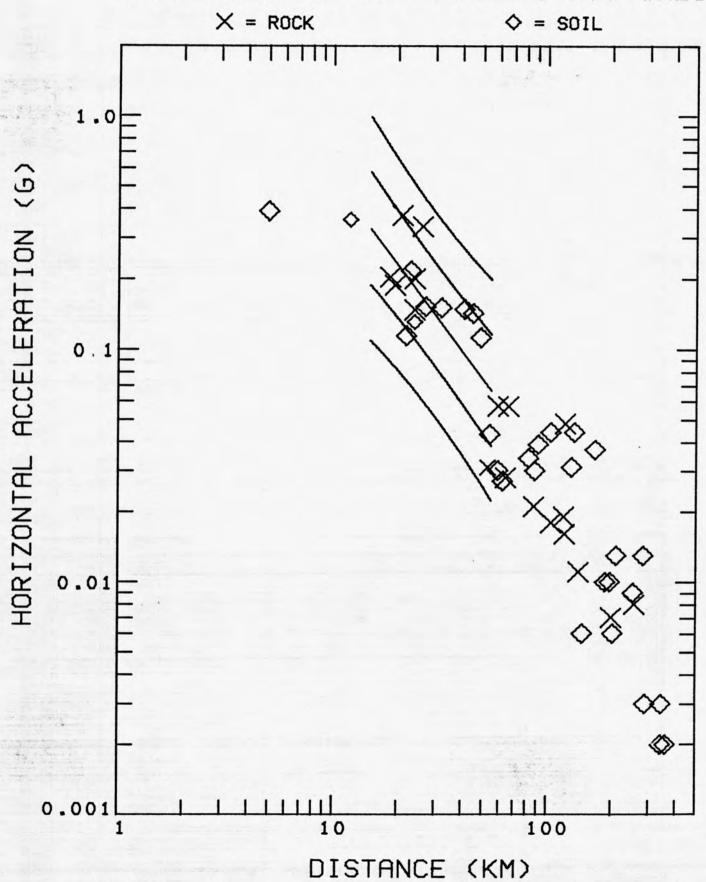
Figure 1. Peak horizontal acceleration versus distance to the slipped fault for the magnitude range 5.0-5.7 recorded at the base of small structures. The x's represent rock sites and the diamonds soil sites. The center line is the mean regression line. The outer pair of lines represents the 95 percent prediction interval, and the inner pair represents the 70 percent prediction interval. Length of lines represents the distance interval considered in the regression analysis.

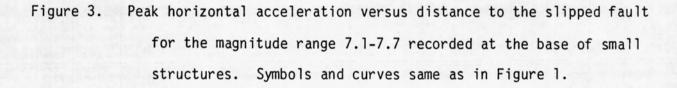
MAGNITUDE 5.0-5.7 SMALL STRUCTURES



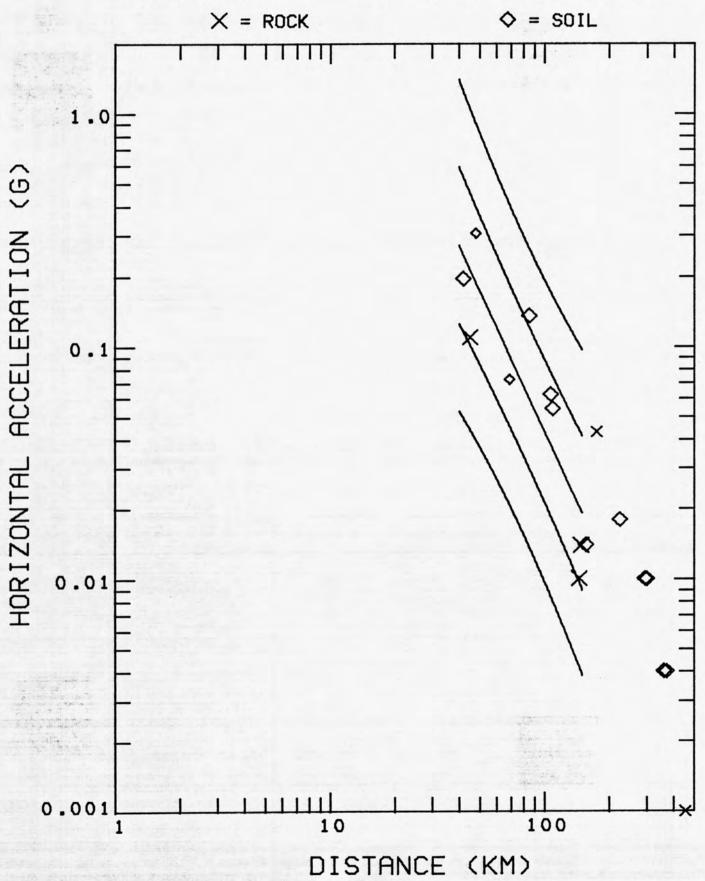


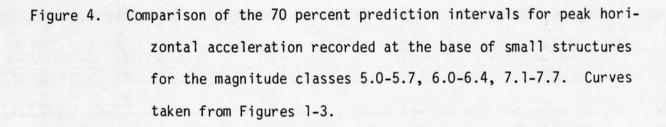
MAGNITUDE 6.0-6.4 SMALL STRUCTURES

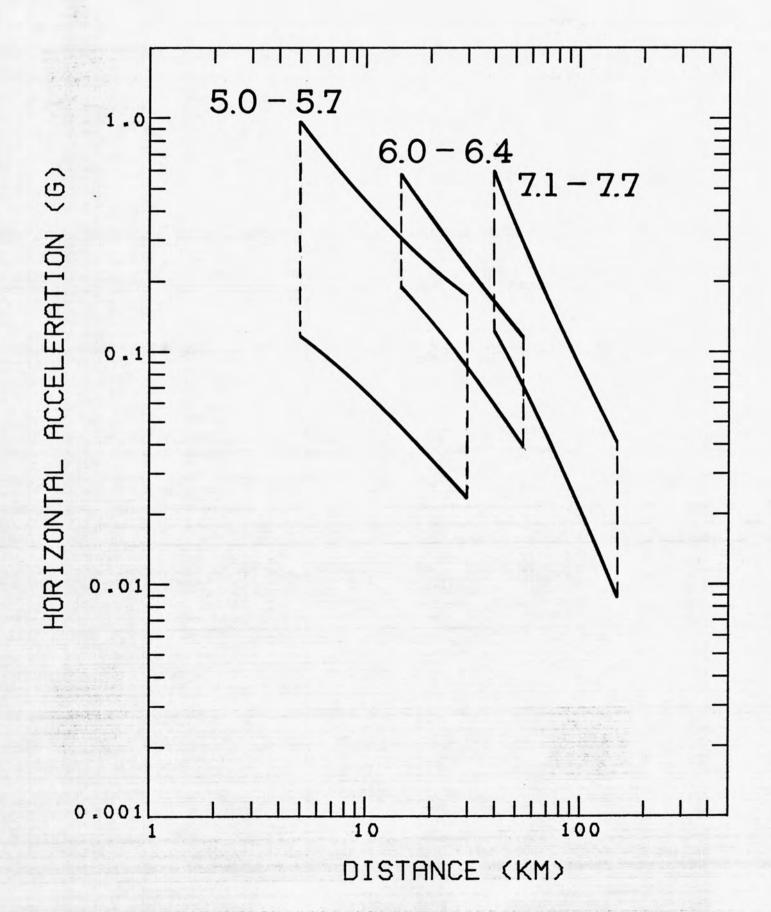


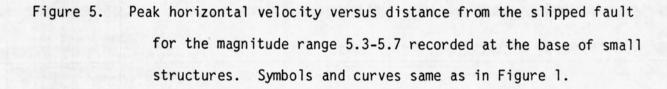


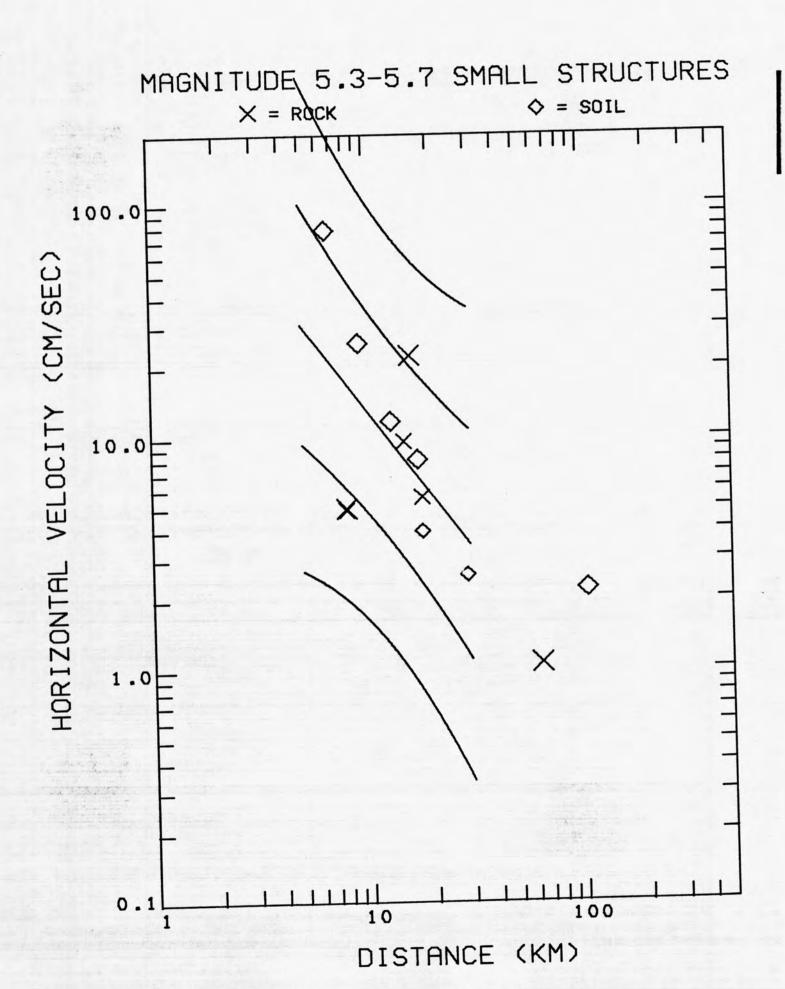
MAGNITUDE 7.1-7.7 SMALL STRUCTURES

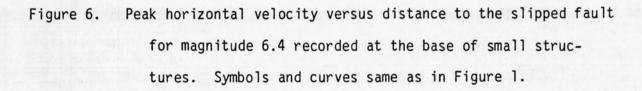












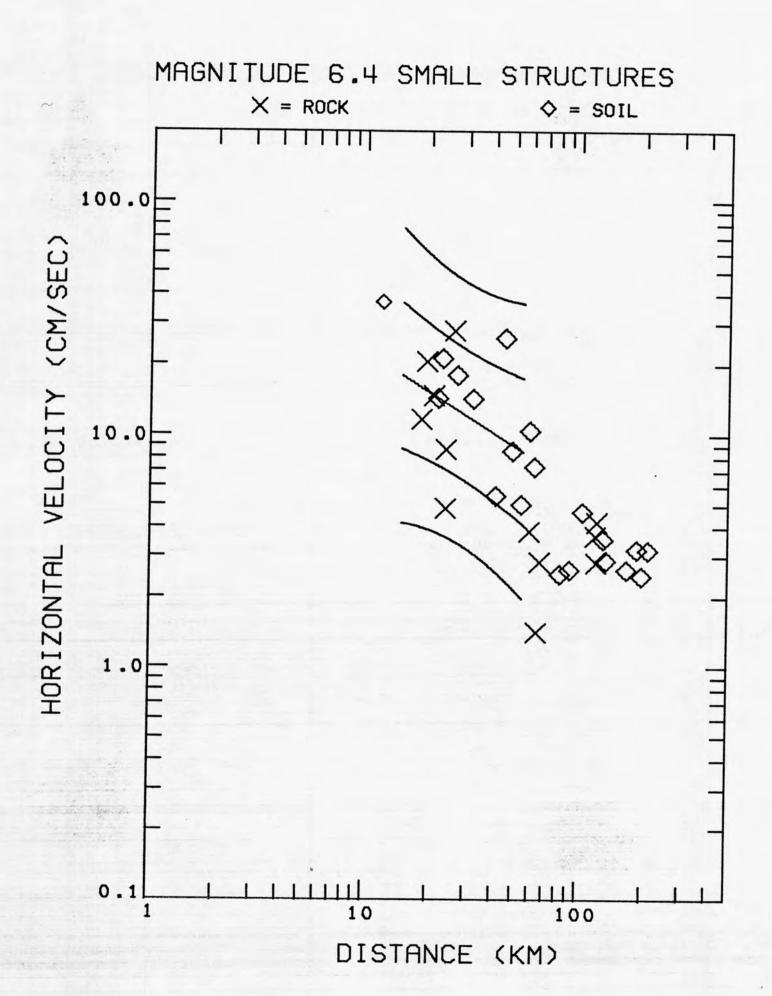


Figure 7. Peak horizontal velocity versus distance to the slipped fault for the magnitude range 7.1-7.7 recorded at the base of small structures. Symbols as in Figure 1.

MAGNITUDE 7.1-7.7 SMALL STRUCTURES

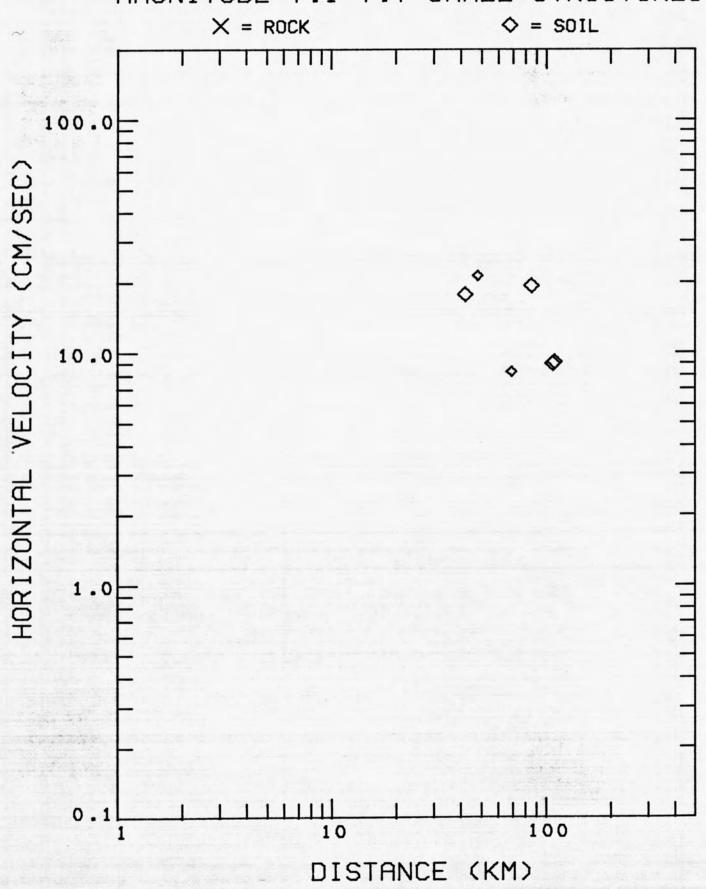
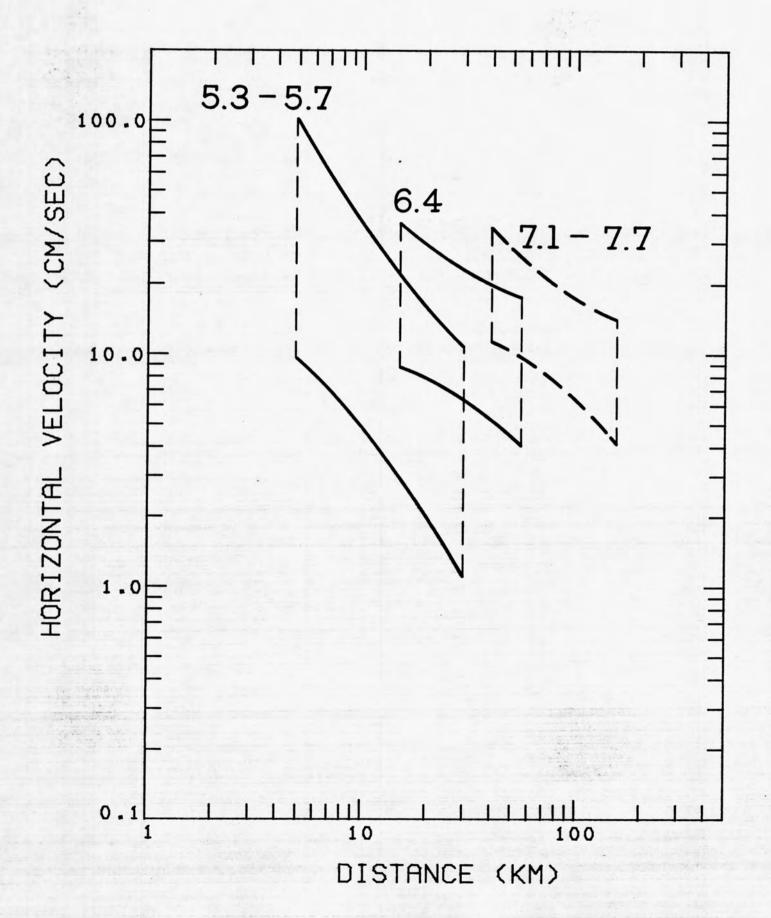
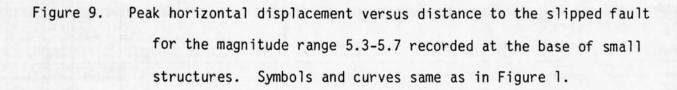


Figure 8. Comparison of the 70 percent prediction intervals for peak horizontal velocity recorded at the base of small structures for the three magnitude classes 5.3-5.7, 6.4, and 7.1-7.7.

Curves for magnitude classes 5.3-5.7 and 6.4 taken from Figures 6 and 7.





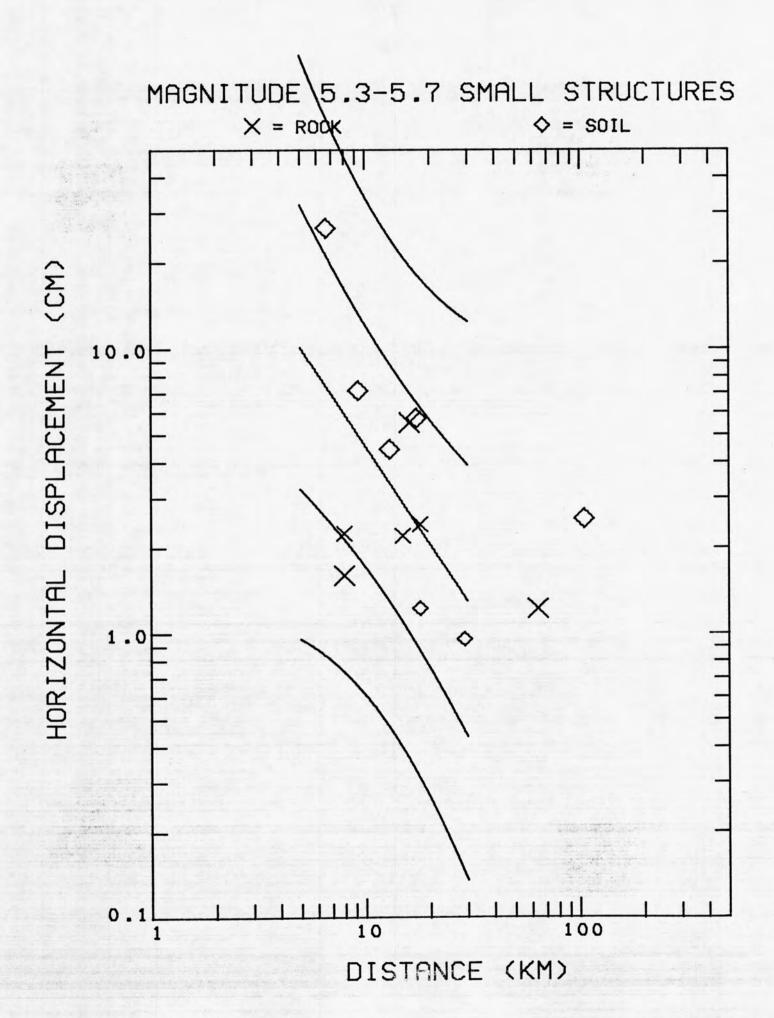


Figure 10. Peak horizontal displacement versus distance to the slipped fault for magnitude 6.4 recorded at the base of small structures. Symbols and curves same as in Figure 1.

MAGNITUDE 6.4 SMALL STRUCTURES

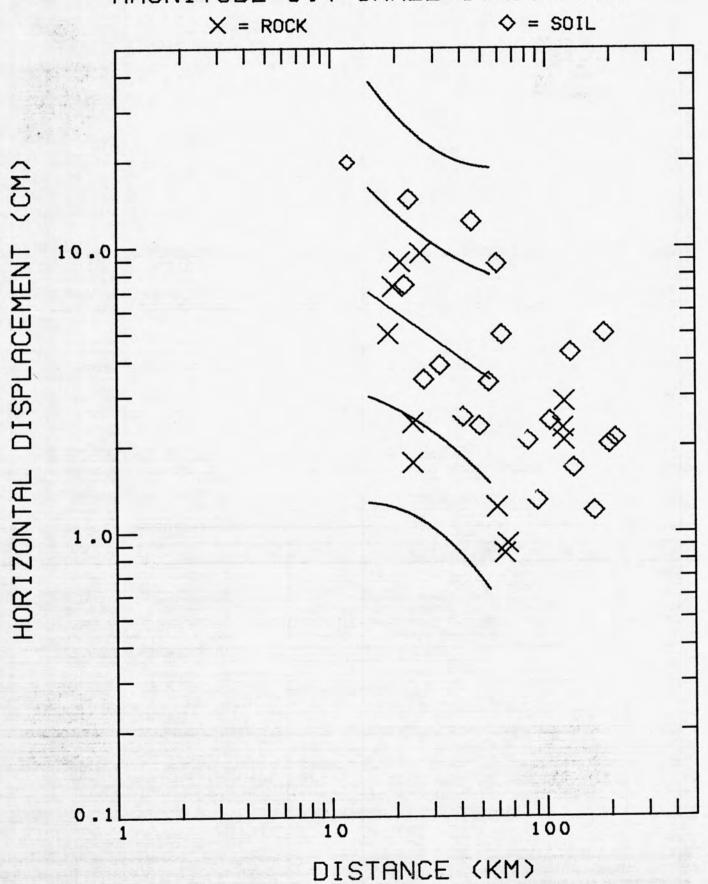
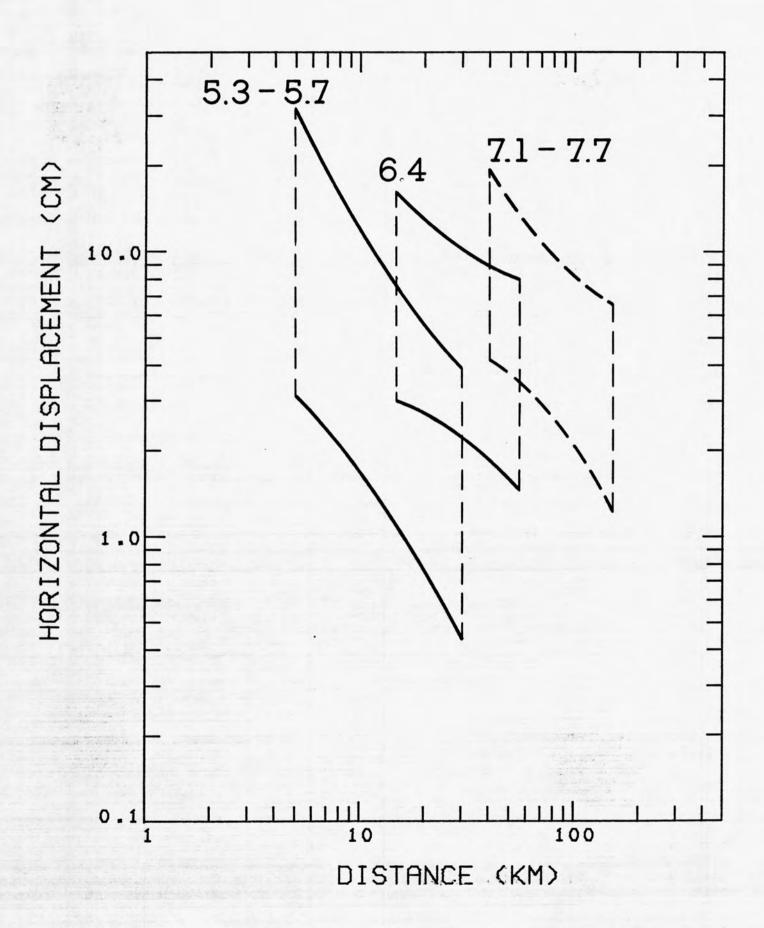


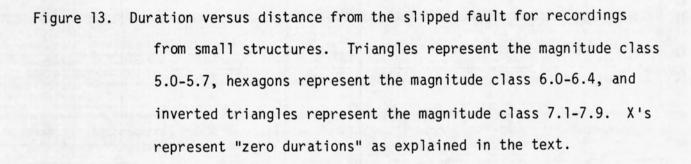
Figure 11. Peak horizontal displacement versus distance to the slipped fault for the magnitude range 7.1-7.7 recorded at the base of small structures. Symbols same as in Figure 1.

MAGNITUDE 7.1-7.7 SMALL STRUCTURES X = ROCK HORIZONTAL DISPLACEMENT (CM) 10.0 1.0 100 10 DISTANCE (KM)

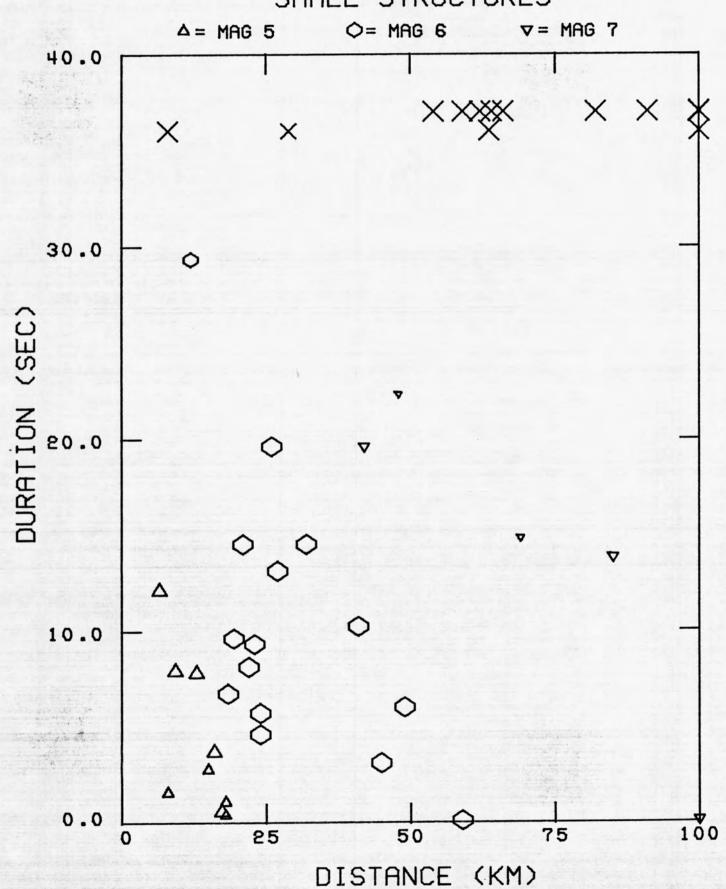
Figure 12. Comparison of the 70 percent prediction intervals for peak horizontal displacement recorded at the base of small structures for the three magnitude classes 5.3-5.7, 6.4, and 7.1-7.7.

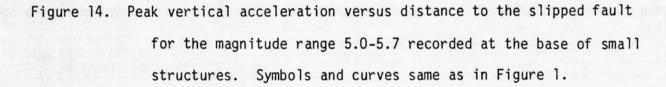
Curves for magnitude classes 5.3-5.7 and 6.4 taken from Figures 11 and 12.





SMALL STRUCTURES





MAGNITUDE 5.0-5.7 SMALL STRUCTURES

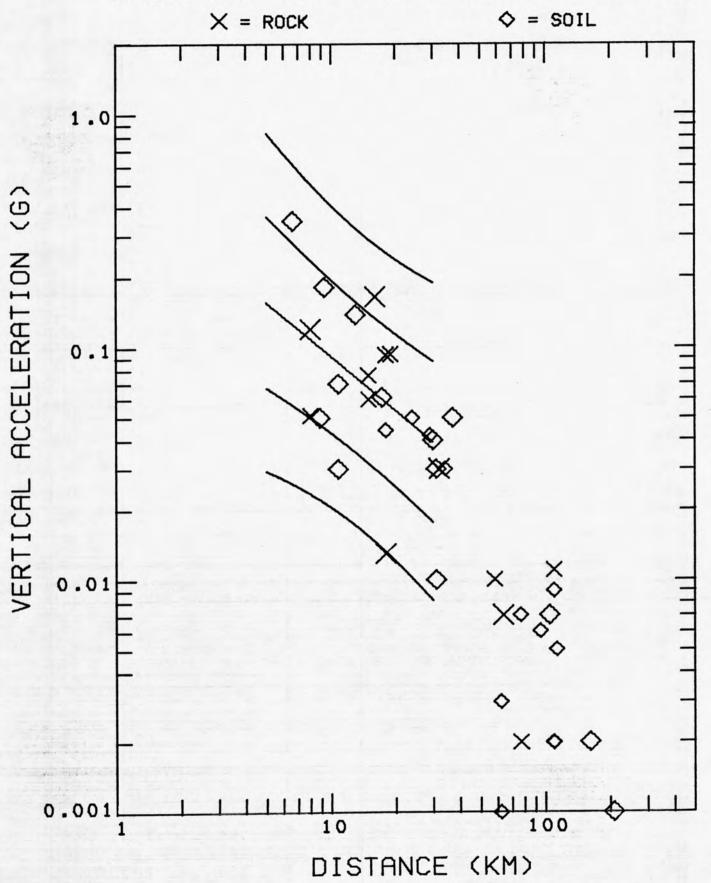


Figure 15. Peak vertical acceleration versus distance to the slipped fault for the magnitude range 6.0-6.4 recorded at the base of small structures. Symbols and curves same as in Figure 1.

MAGNITUDE 6.0-6.4 SMALL STRUCTURES X = ROCK 1111111 1.0 VERTICAL ACCELERATION (3) 0.1 0.01 0.001 100

10

DISTANCE (KM)

Figure 16. Peak vertical acceleration versus distance to the slipped fault for the magnitude range 7.1-7.7 recorded at the base of small structures. Symbols and curves same as in Figure 1.

MAGNITUDE 7.1-7.7 SMALL STRUCTURES

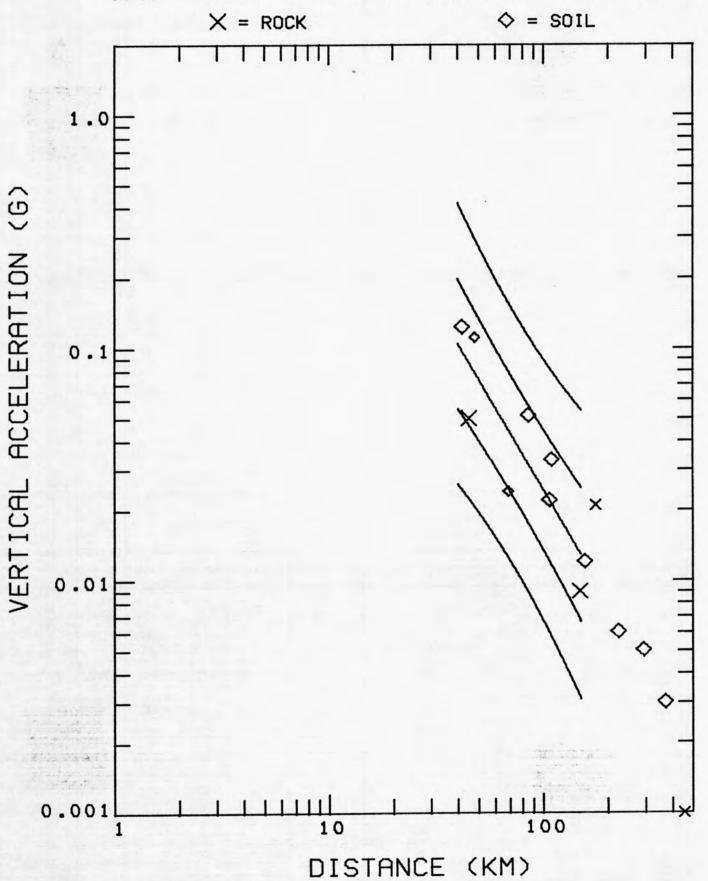


Figure 17. Peak vertical velocity versus distance to the slipped fault for the magnitude range 5.3-5.7 recorded at the base of small structures. Symbols and curves same as in Figure 1.

MAGNITUDE 5.3-5.7 SMALL STRUCTURES

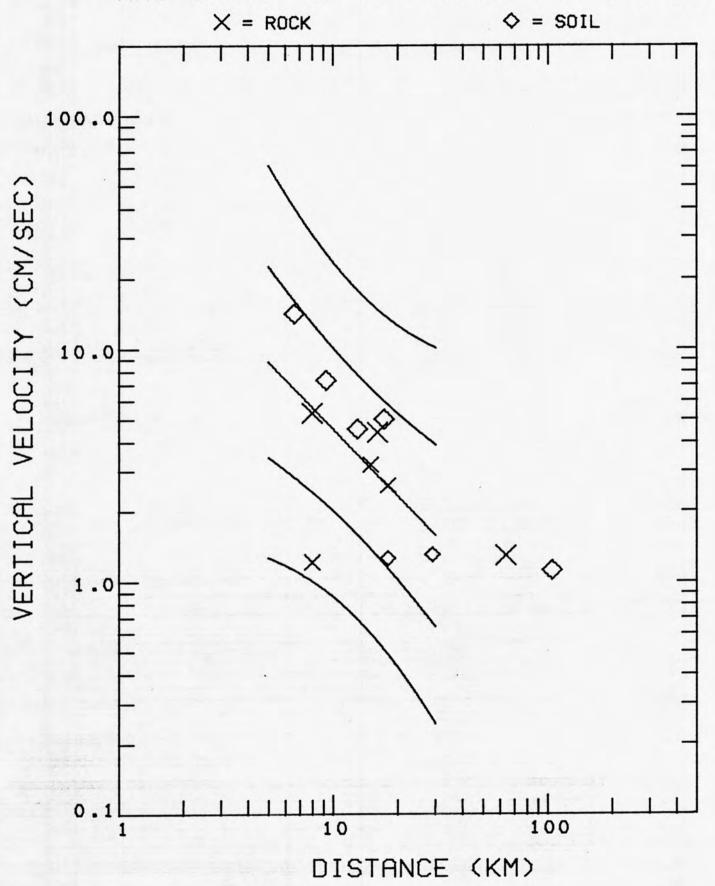


Figure 18. Peak vertical velocity versus distance to the slipped fault for magnitude 6.4 recorded at the base of small structures. Symbols and curves same as in Figure 1.

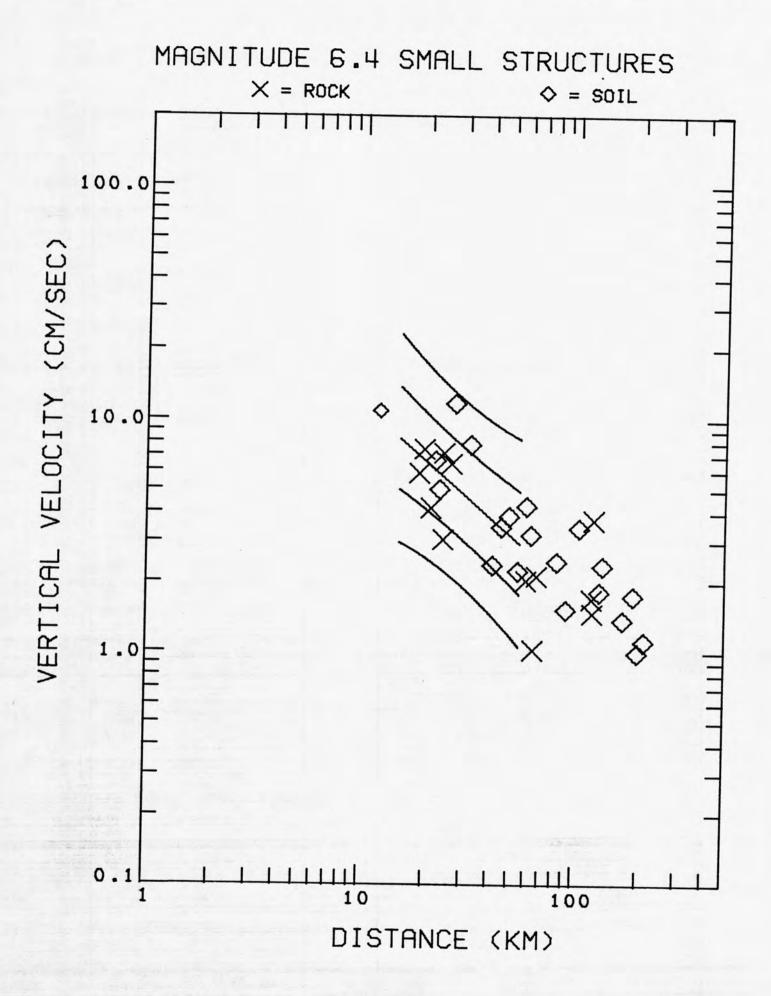


Figure 19. Peak vertical velocity versus distance to the slipped fault for the magnitude range 7.1-7.7 recorded at the base of small structures. Symbols same as in Figure 1.

MAGNITUDE 7.1-7.7 SMALL STRUCTURES

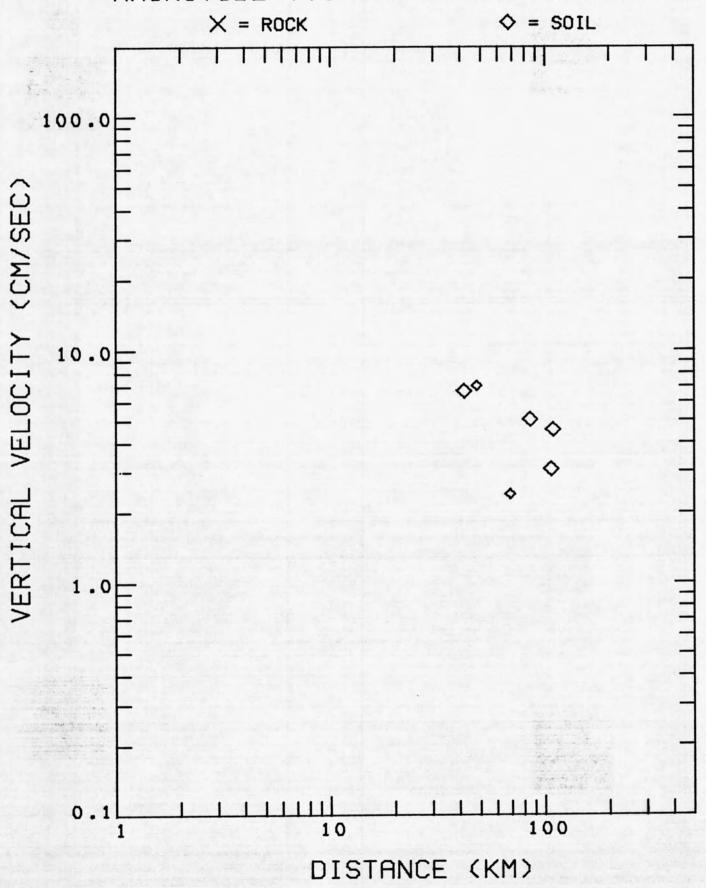


Figure 20. Peak vertical displacement versus distance to the slipped fault for the magnitude range 5.3-5.7 recorded at the base of small structures. Symbols and curves same as in Figure 1.

MAGNITUDE 5.3-5.7 SMALL STRUCTURES

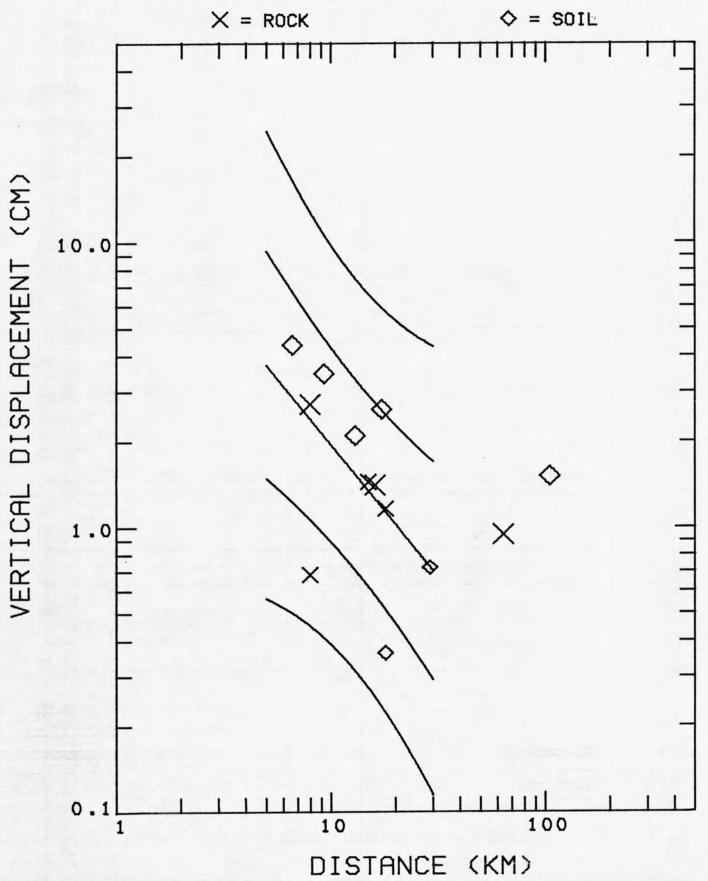


Figure 21. Peak vertical displacement versus distance to the slipped fault for magnitude 6.4 recorded at the base of small structures. Symbols and curves same as in Figure 1.

MAGNITUDE 6.4 SMALL STRUCTURES X = ROCK VERTICAL DISPLACEMENT (CM) 10.0 1.0 X 100

DISTANCE (KM)

Figure 22. Peak vertical displacement versus distance from the slipped fault for the magnitude range 7.1-7.7 recorded at the base of small structures. Symbols same as in Figure 1.

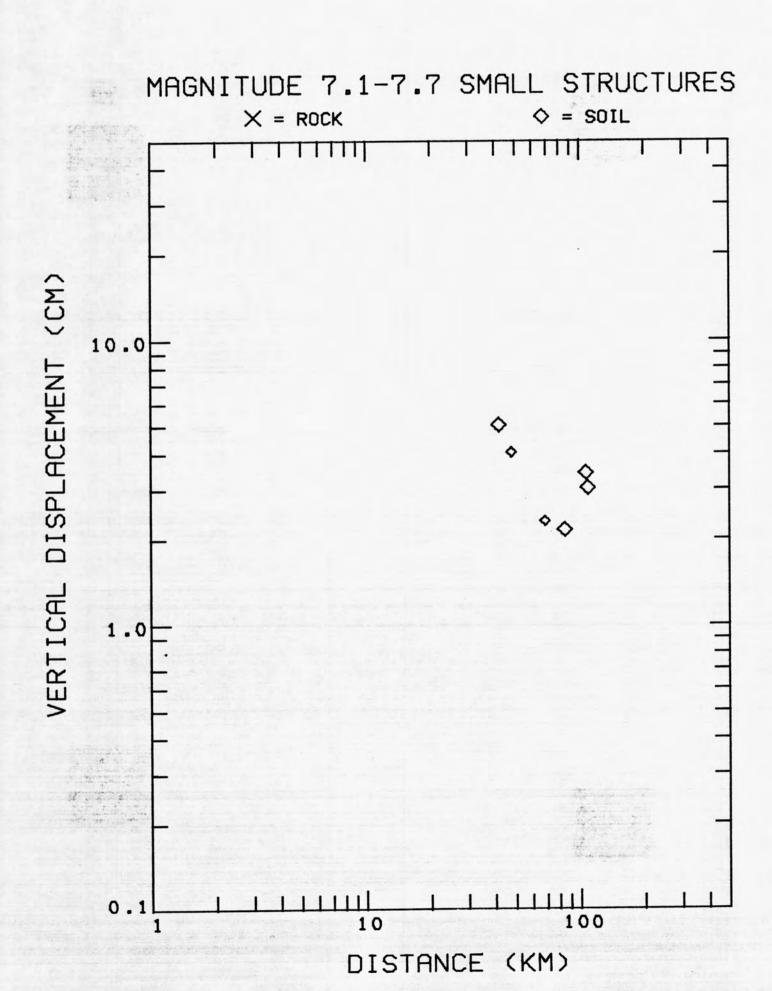


Figure 23. Peak horizontal acceleration versus distance to the slipped fault for the magnitude range 5.0-5.7 including data from both large and small structures. The pluses represent rock sites and the squares soil sites. The center line is the mean regression line. The outer pair of lines represents the 95 percent prediction interval, and the inner pair represents the 70 percent prediction interval. Length of lines represents the distance interval considered in the regression analysis.

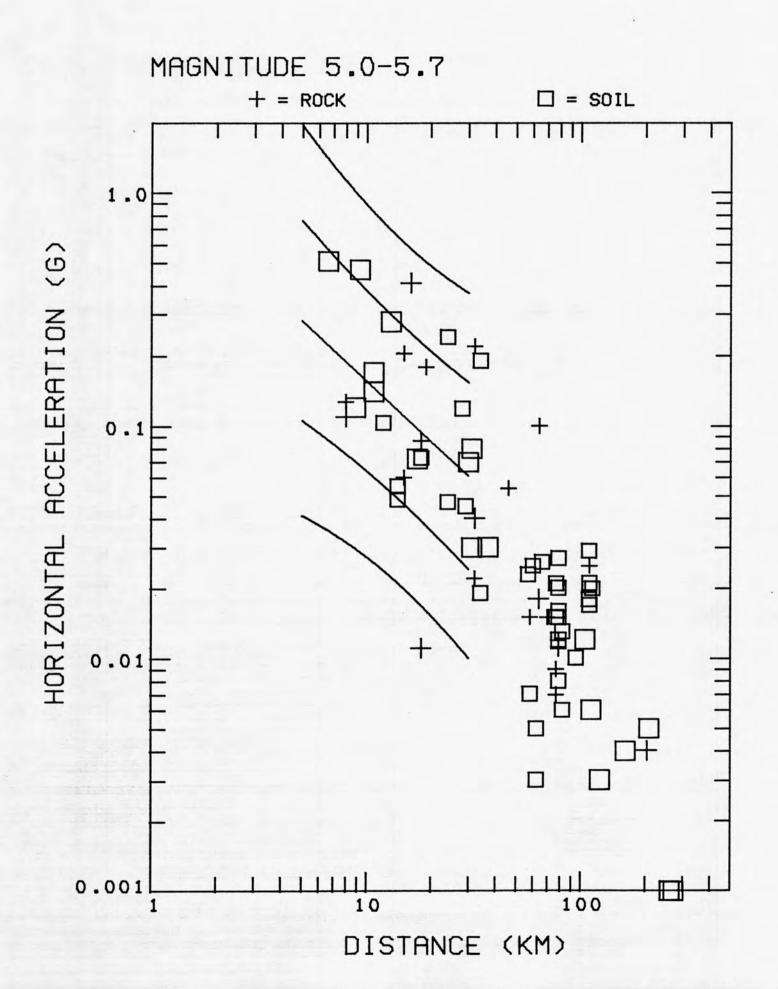


Figure 24. Peak horizontal acceleration versus distance to the slipped fault for the magnitude range 6.0-6.4 including data from both large and small structures. Symbols and curves same as in Figure 23.

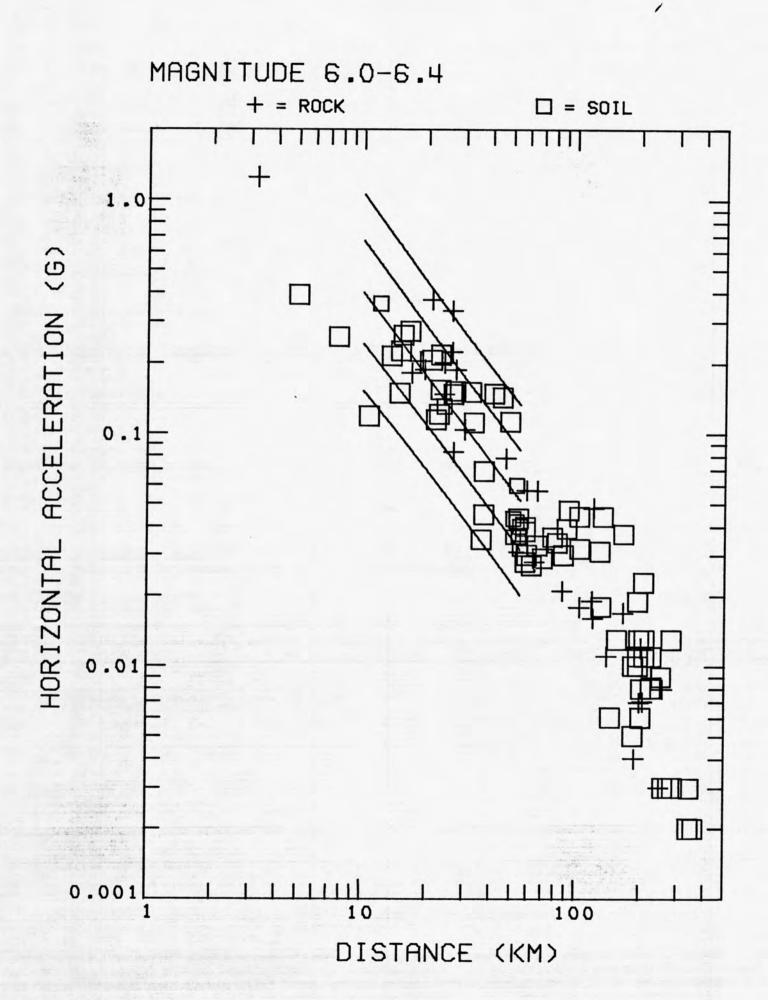


Figure 25. Peak horizontal acceleration versus distance to the slipped fault for the magnitude range 7.1-7.7 including data from both large and small structures. Symbols and curves same as in Figure 23.

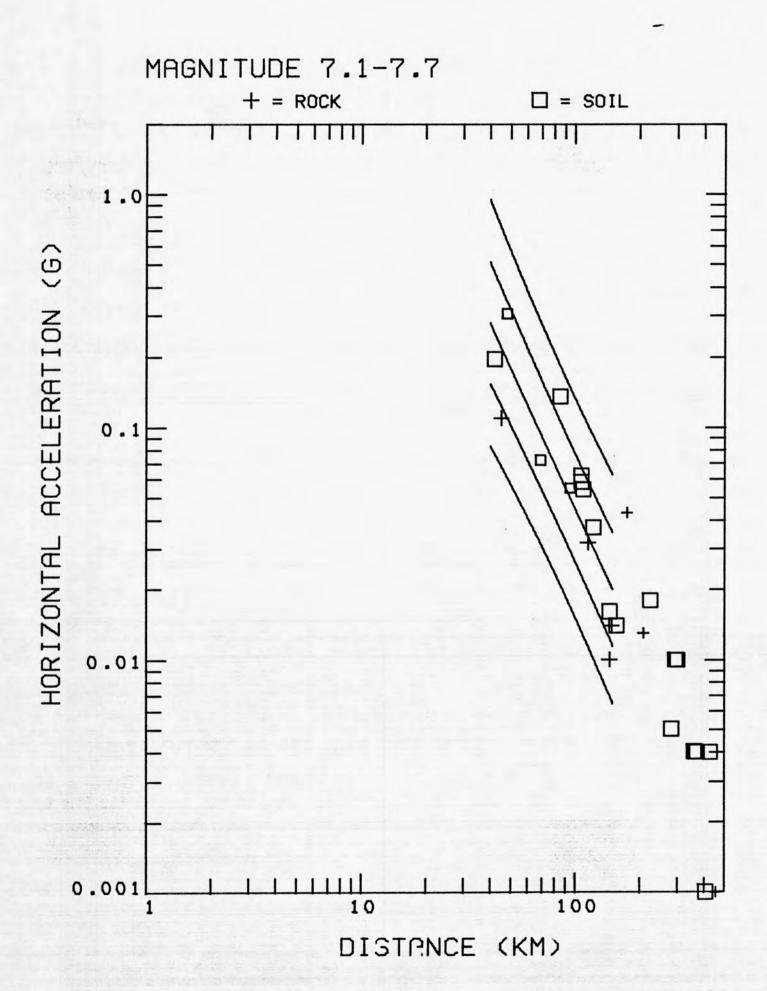


Figure 26. Peak horizontal velocity versus distance to the slipped fault for the magnitude range 5.3-5.7 including data from both large and small structures. Symbols and curves same as in Figure 23.

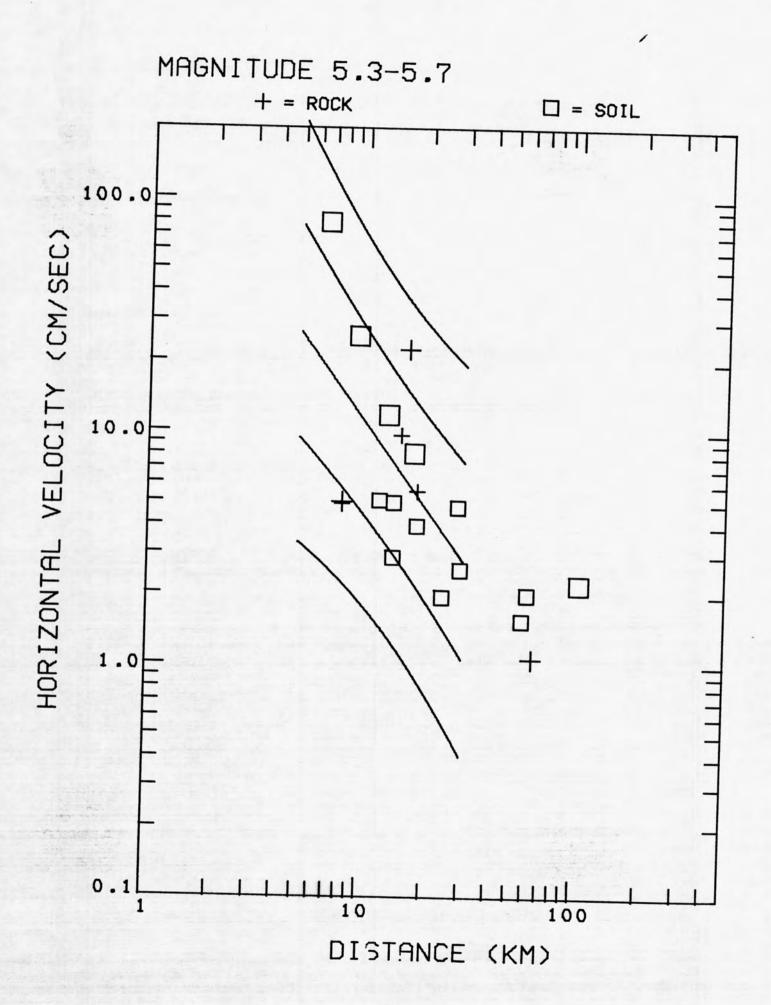


Figure 27. Peak horizontal velocity versus distance to the slipped fault for magnitude 6.4 including data from both large and small structures. Symbols and curves same as in Figure 23.

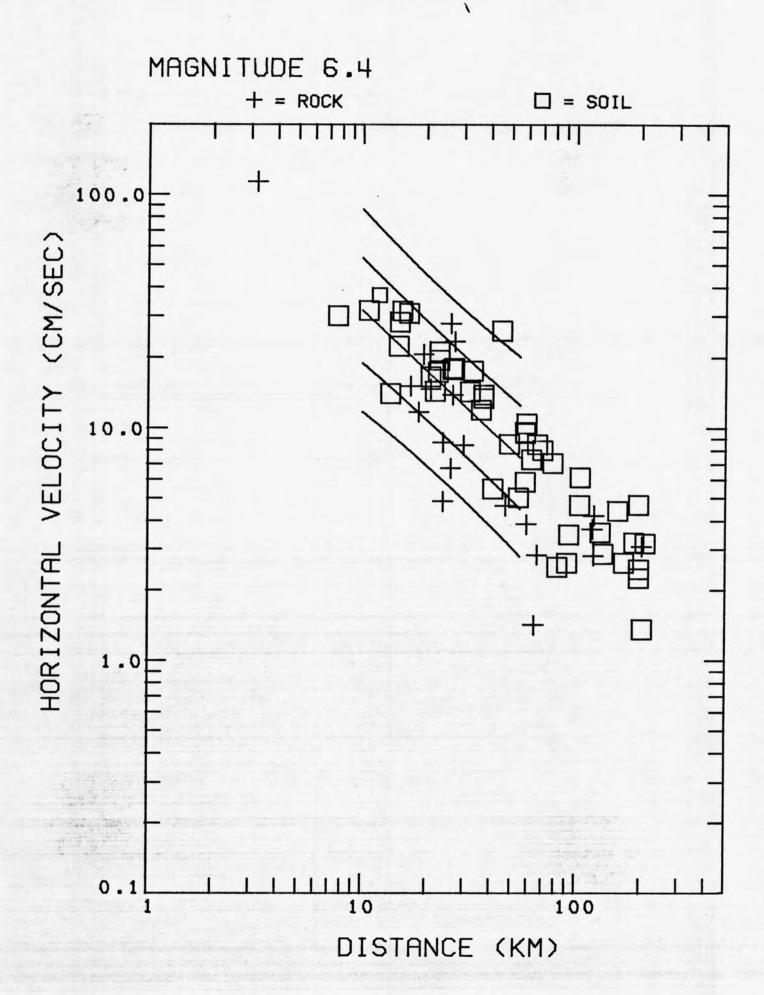


Figure 28. Peak horizontal velocity versus distance to the slipped fault for the magnitude range 7.1-7.7 including data from both large and small structures. Symbols same as in Figure 23.

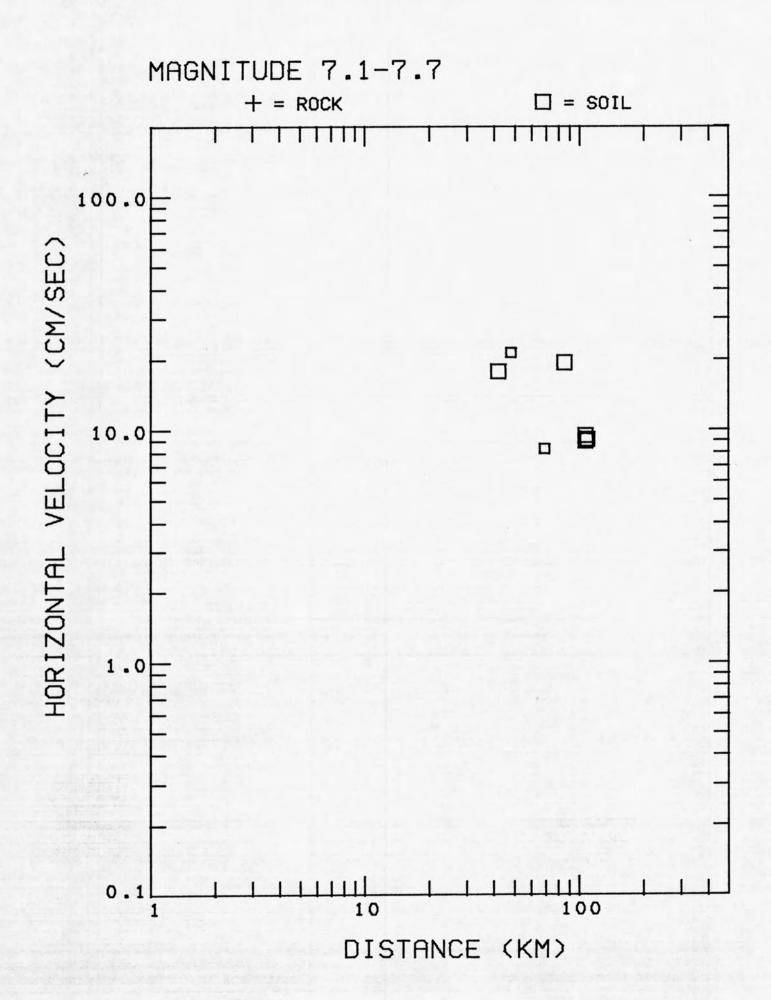


Figure 29. Peak horizontal displacement versus distance to the slipped fault for the magnitude range 5.3-5.7 including data from both large and small structures. Symbols and curves same as in Figure 23.

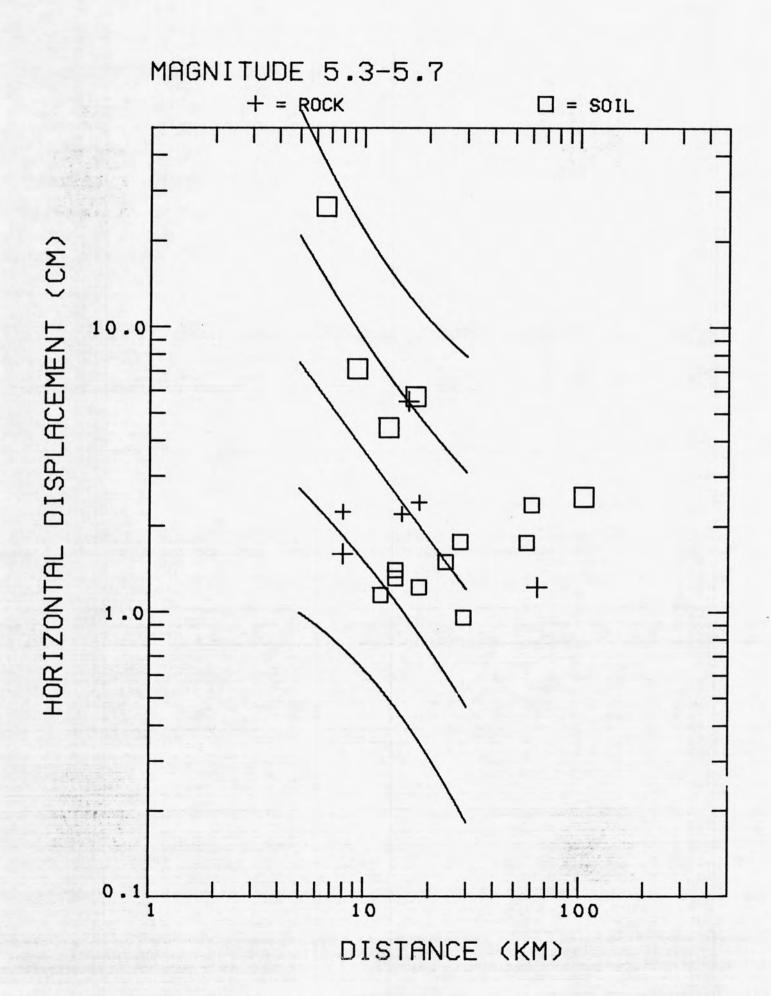


Figure 30. Peak horizontal displacement versus distance to the slipped fault for magnitude 6.4 including data from both large and small structures. Symbols and curves same as in Figure 23.

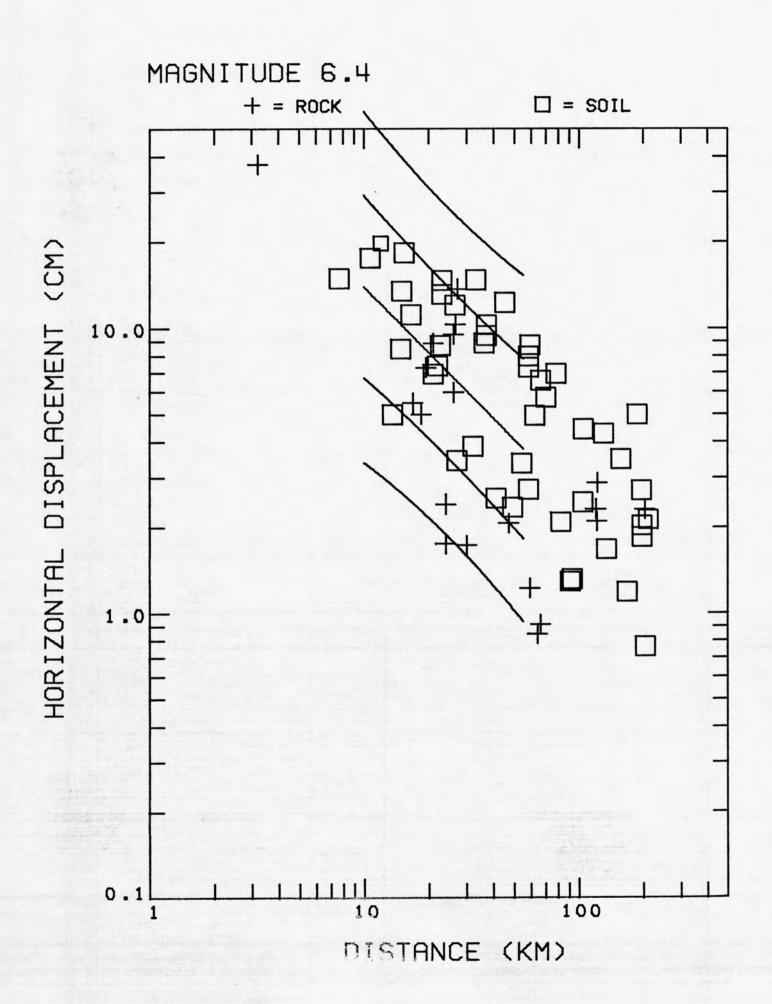
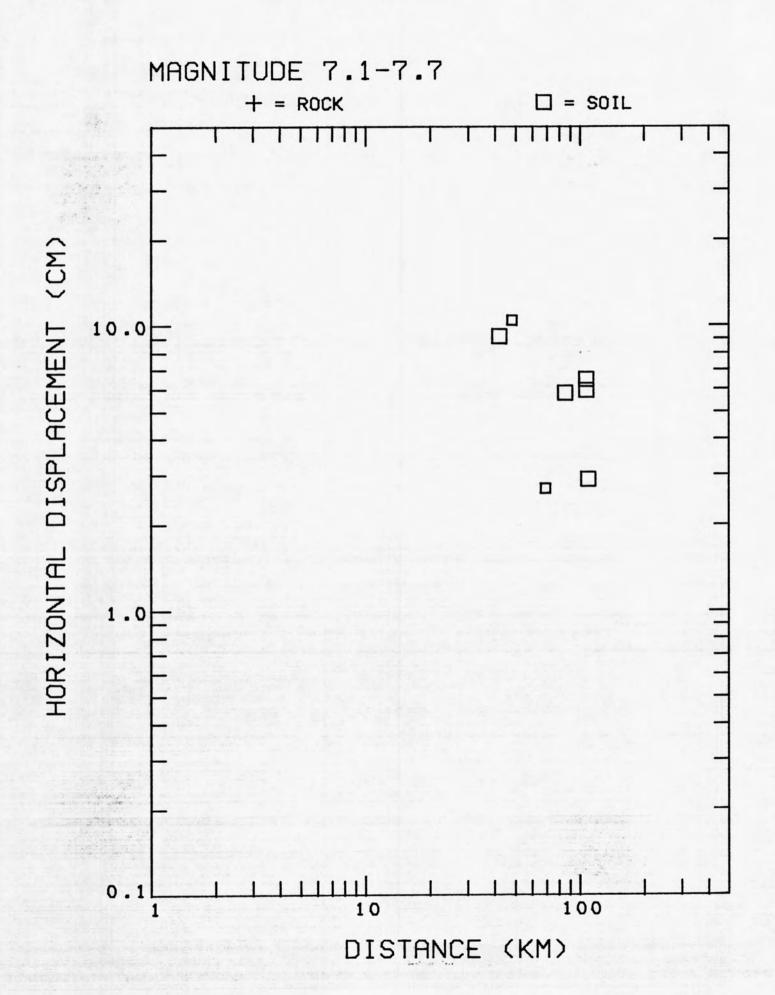
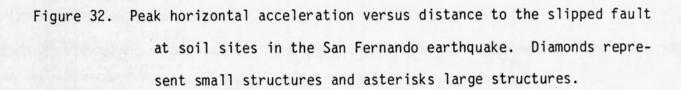


Figure 31. Peak horizontal displacement versus distance to the slipped fault for the magnitude range 7.1-7.7 including data from both large and small structures. Symbols same as in Figure 23.





SAN FERNANDO SOIL SITES

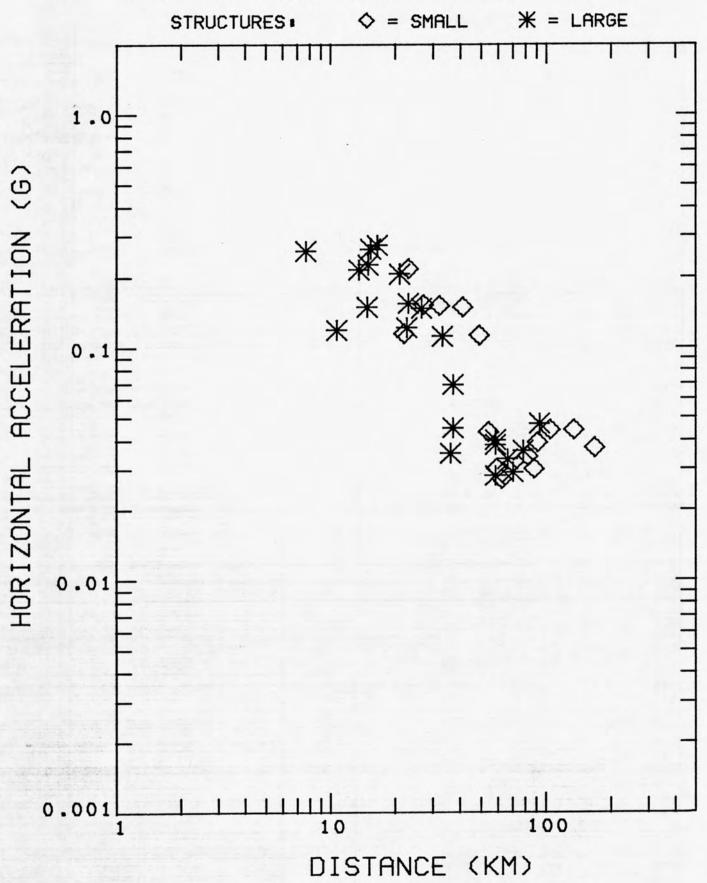


Figure 33. Comparison of mean regression lines and 70 percent prediction intervals for small structures (solid lines) and large structures (dashed lines) for peak horizontal acceleration at soil sites in the San Fernando earthquake.

SAN FERNANDO SOIL SITES SMALL STRUCTURES LARGE STRUCTURES 1.0 HORIZONTAL ACCELERATION (G) 0.1 0.01 0.001 100 DISTANCE (KM)

Figure 34. Peak horizontal velocity versus distance to the slipped fault at soil sites in the San Fernando earthquake. Diamonds represent small structures and asterisks large structures.

SAN FERNANDO SOIL SITES

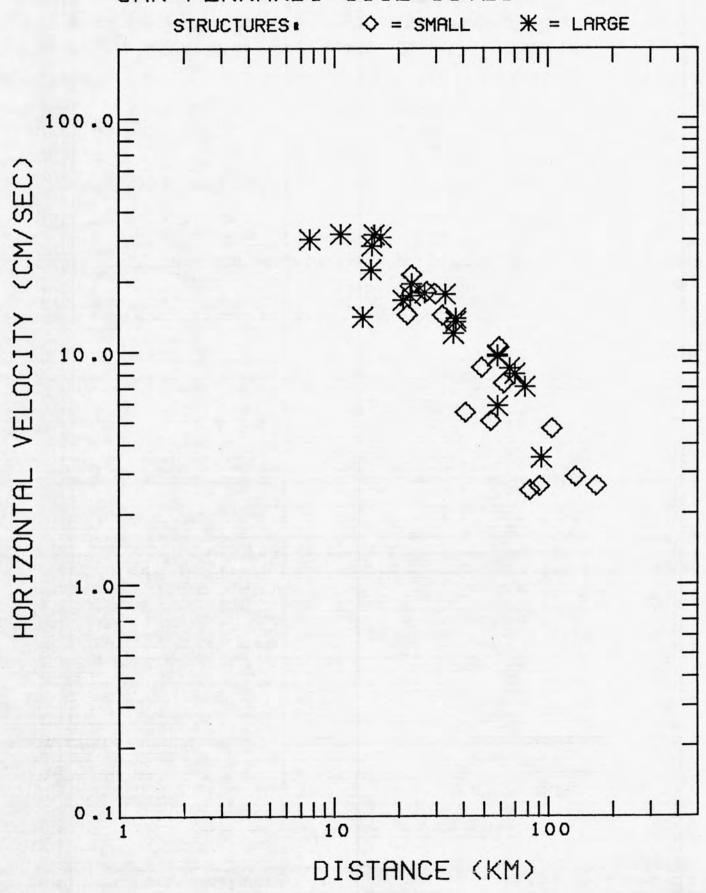


Figure 35. Comparison of mean regression lines and 70 percent prediction intervals for small structures (solid lines) and large structures (dashed lines) for peak horizontal velocity at soil sites in the San Fernando earthquake.

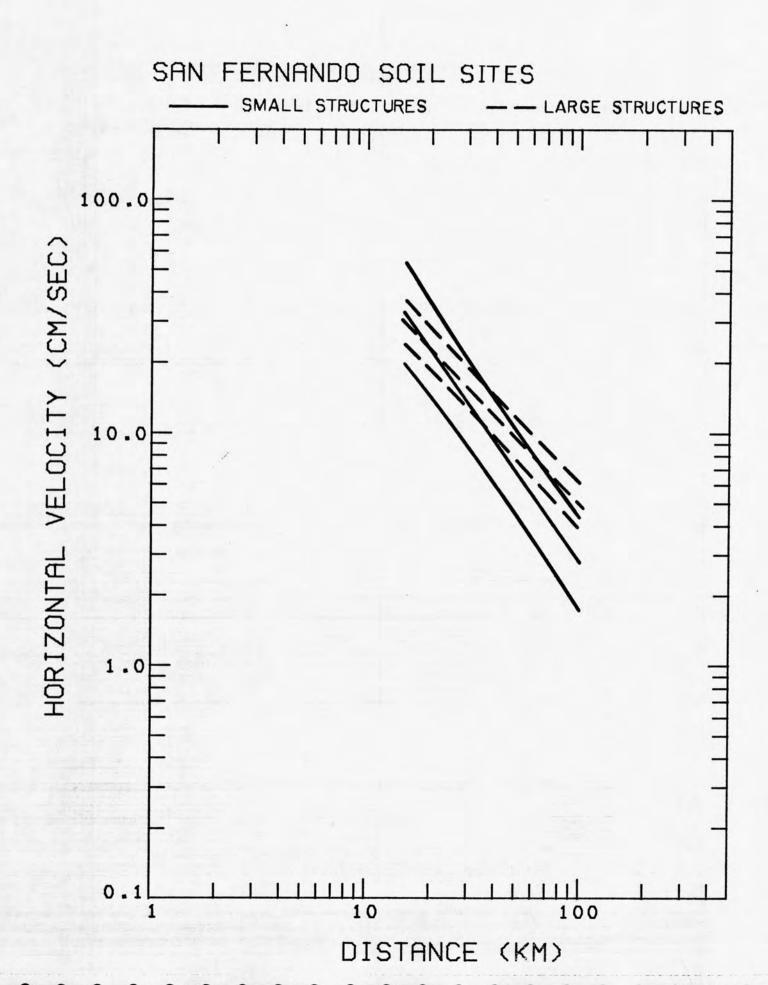


Figure 36. Peak horizontal displacement versus distance to the slipped fault at soil sites in the San Fernando earthquake. Diamonds represent small structures and asterisks large structures.

SAN FERNANDO SOIL SITES

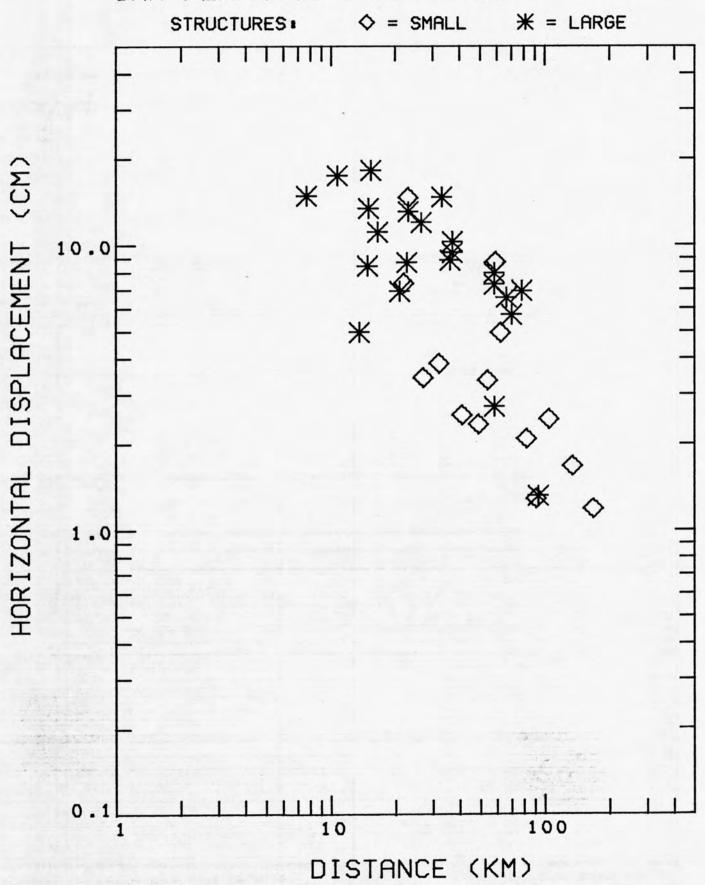


Figure 37. Comparison of mean regression lines and 70 percent prediction intervals for small structures (solid lines) and large structures (dashed lines) for peak horizontal displacement at soil sites in the San Fernando earthquake.

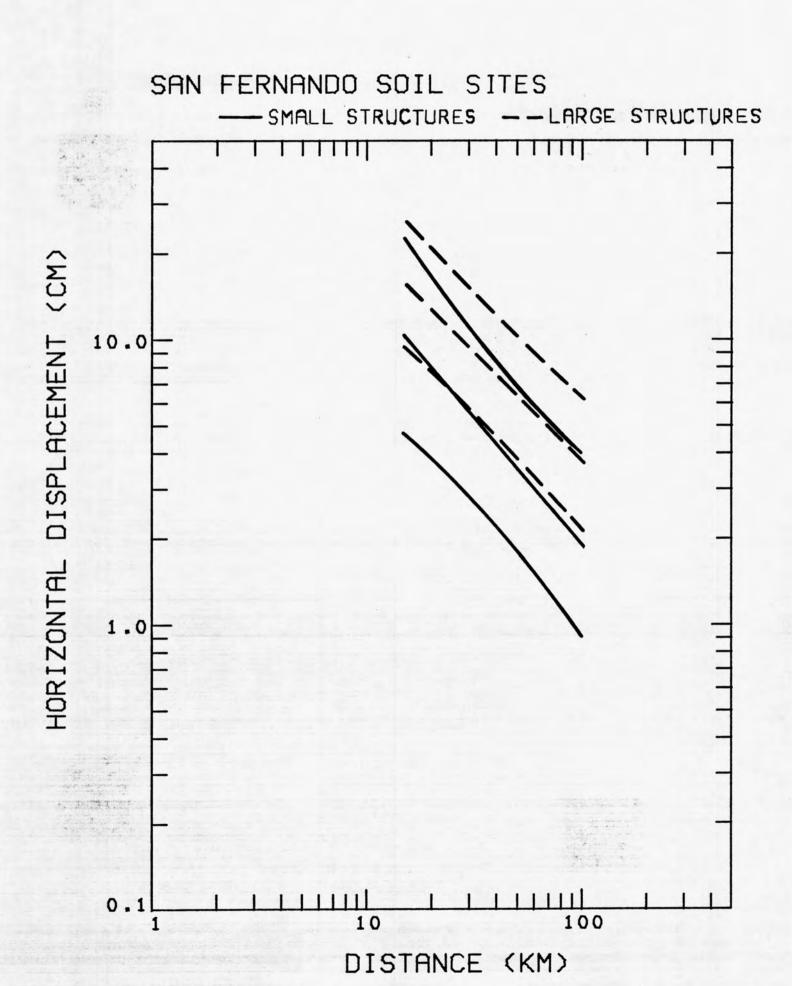


Figure 38. Peak horizontal acceleration recorded at the base of small structures versus distance to the slipped fault in the San Fernando earthquake. Symbols same as in Figure 1.

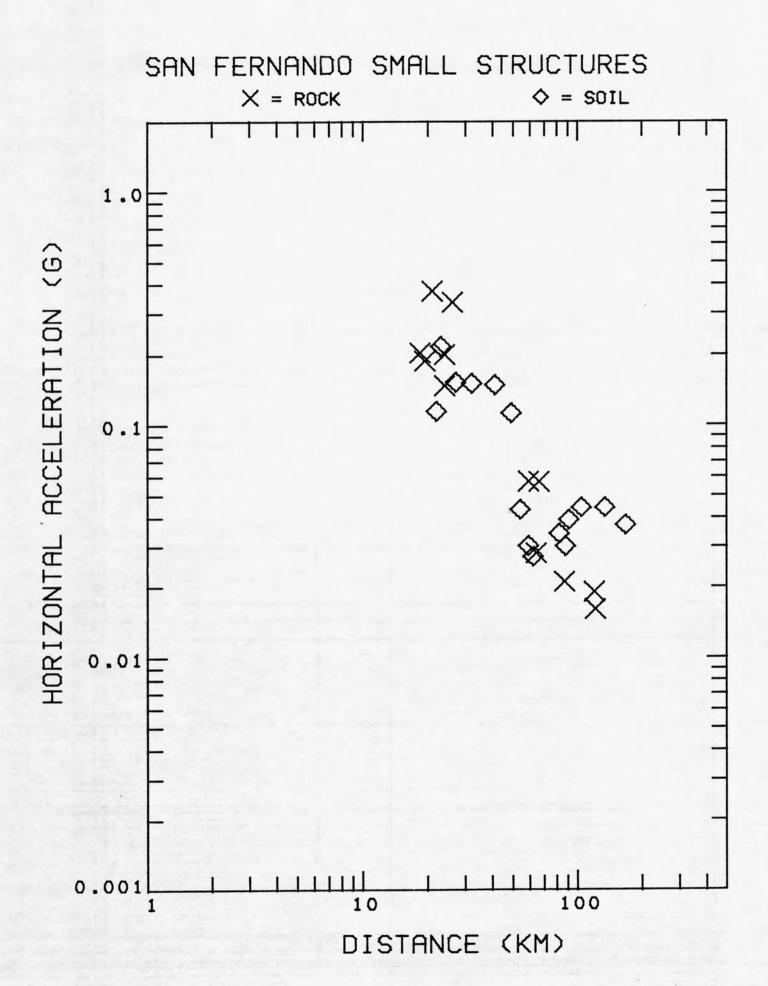


Figure 39. Comparison of mean regression lines and 70 percent prediction intervals for rock sites (solid lines) and soil sites (dashed lines) for peak horizontal acceleration recorded at the base of small structures in the San Fernando earthquake.

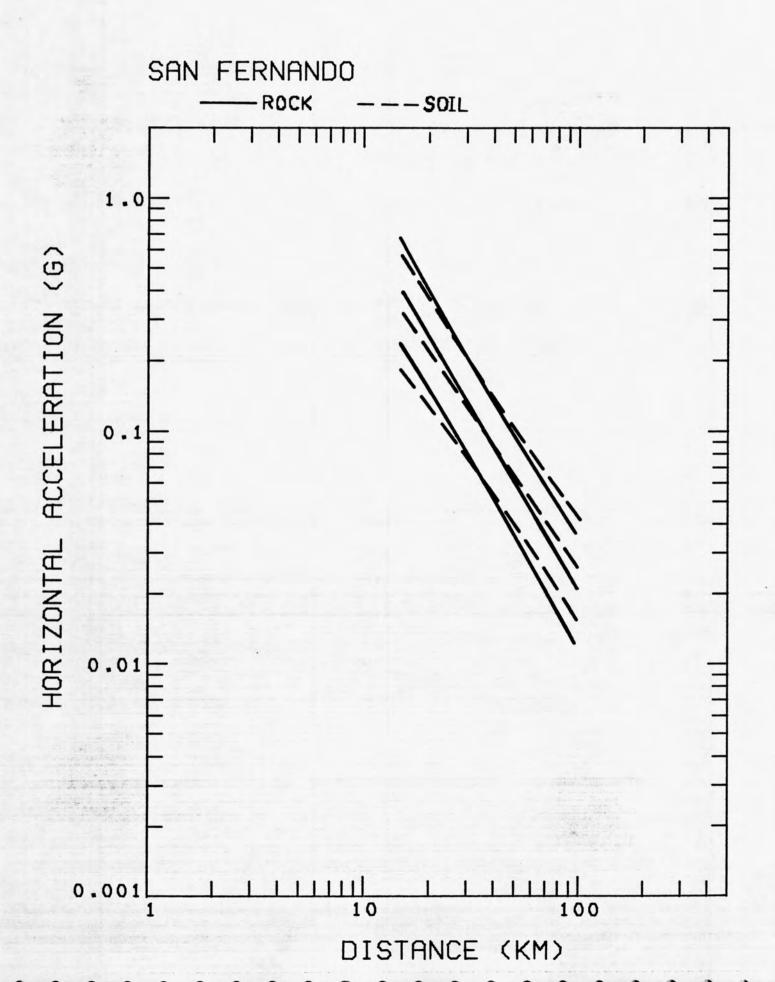


Figure 40. Peak horizontal velocity recorded at the base of small structures versus distance to the slipped fault in the San Fernando earthquake. Symbols same as in Figure 1.

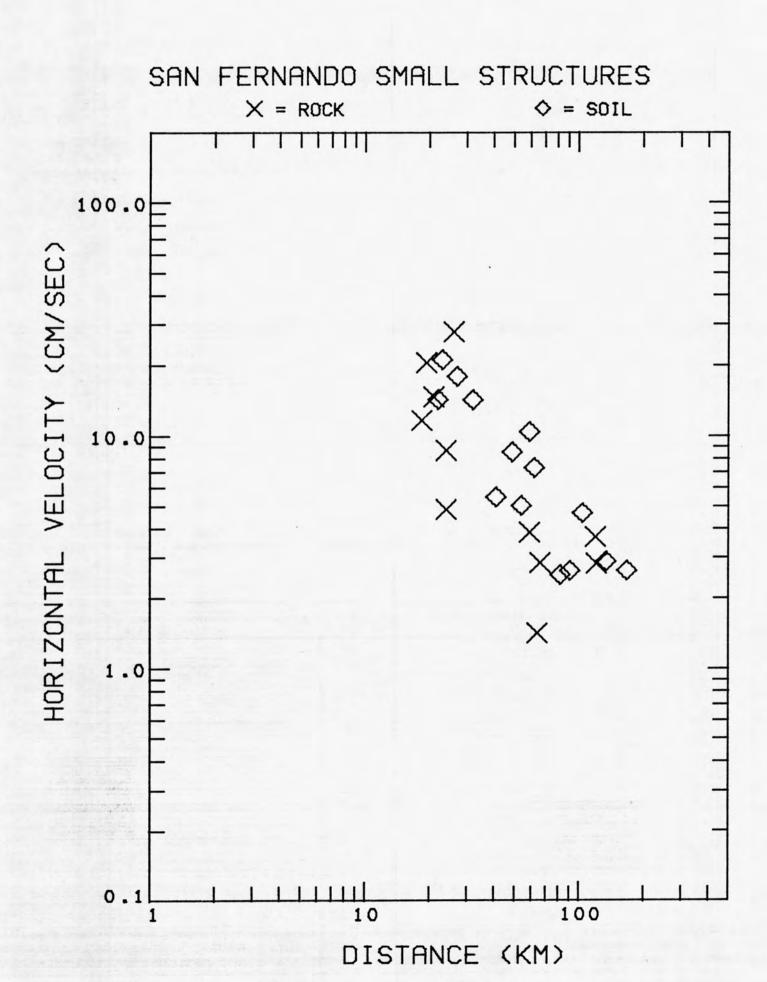


Figure 41. Comparison of mean regression lines and 70 percent prediction intervals for rock sites (solid lines) and soil sites (dashed lines) for peak horizontal velocity recorded at the base of small structures in the San Fernando earthquake.

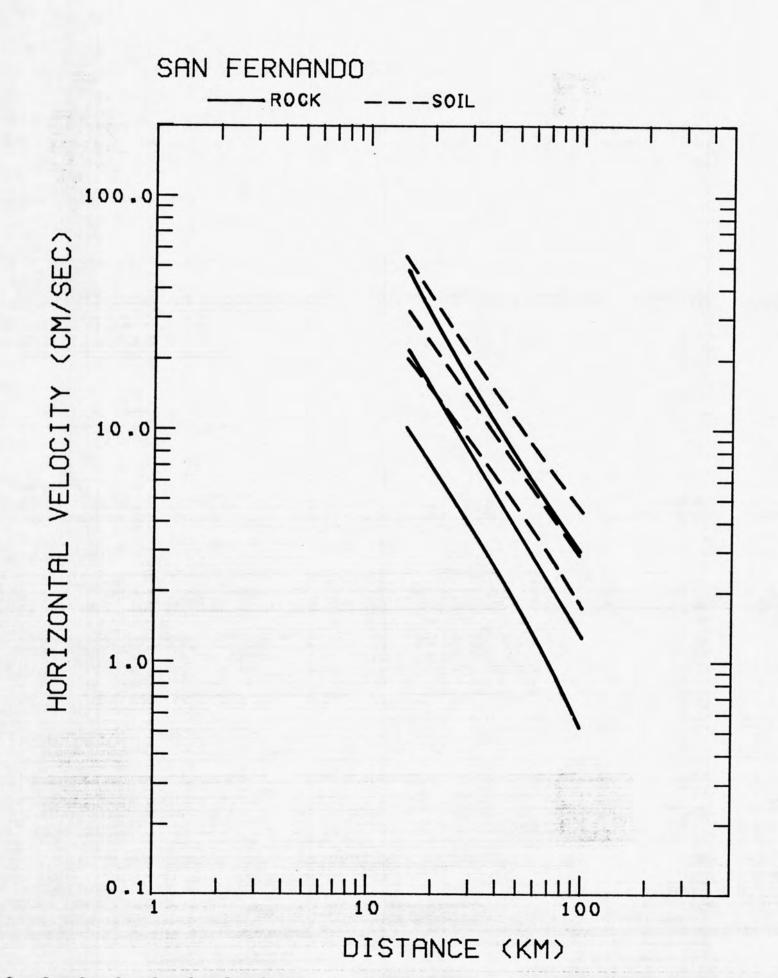


Figure 42. Peak horizontal displacement recorded at the base of small structures versus distance to the slipped fault in the San Fernando earthquake. Symbols same as in Figure 1.

SAN FERNANDO SMALL STRUCTURES X = ROCK 111111 HORIZONTAL DISPLACEMENT (CM) 10.0 1.0 0.1 100

DISTANCE

(KM)

Figure 43. Comparison of mean regression lines and 70 percent prediction intervals for rock sites (solid lines) and soil sites (dashed lines) for peak horizontal displacement recorded at the base of small structures in the San Fernando earthquake.

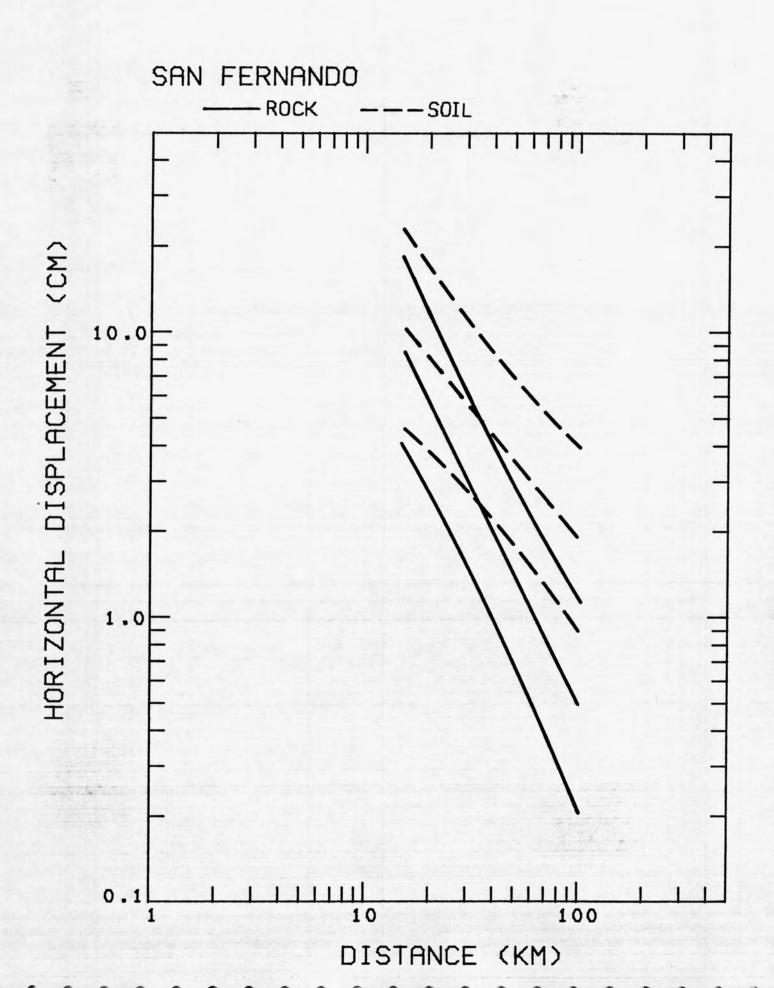


Figure 44. Peak horizontal acceleration in the San Fernando earthquake recorded at the base of small structures. Azimuthal dependence of the residuals from the mean regression line. See text for further explanation. Symbols same as in Figure 1.

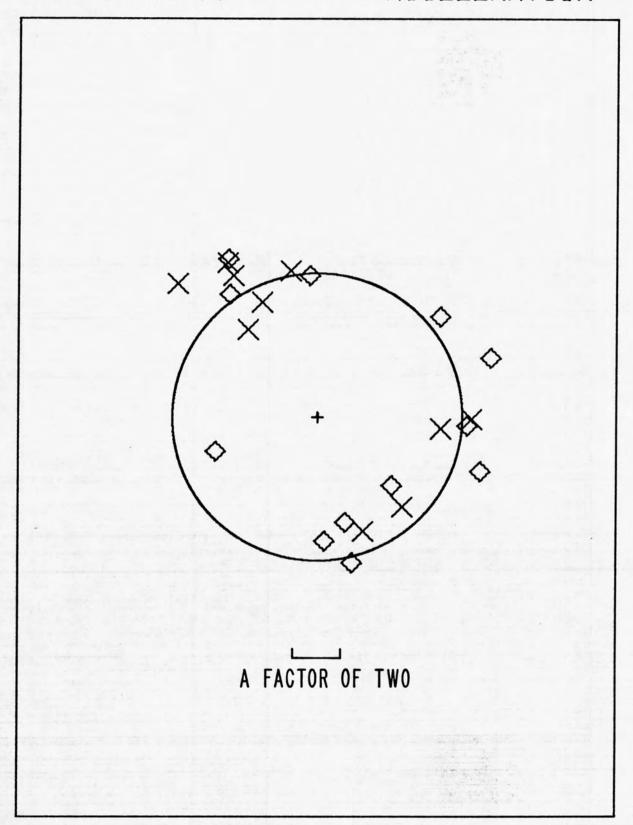


Figure 45. Peak horizontal velocity in the San Fernando earthquake recorded at the base of small structures. Azimuthal dependence of the residuals from the mean regression line. See
text for further explanation. Symbols same as in Figure 1.

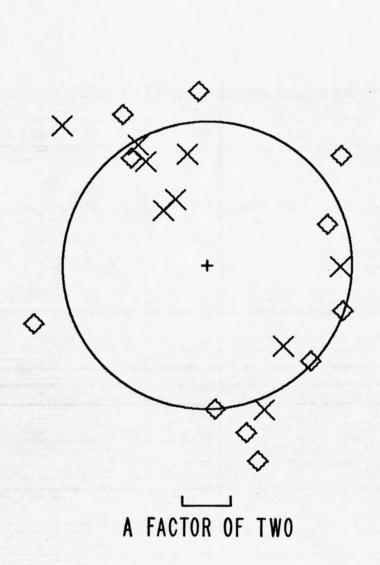


Figure 46. Peak horizontal displacement in the San Fernando earthquake recorded at the base of small structures. Azimuthal dependence of the residuals from the mean regression line. See text for further explanation. Symbols same as in Figure 1.

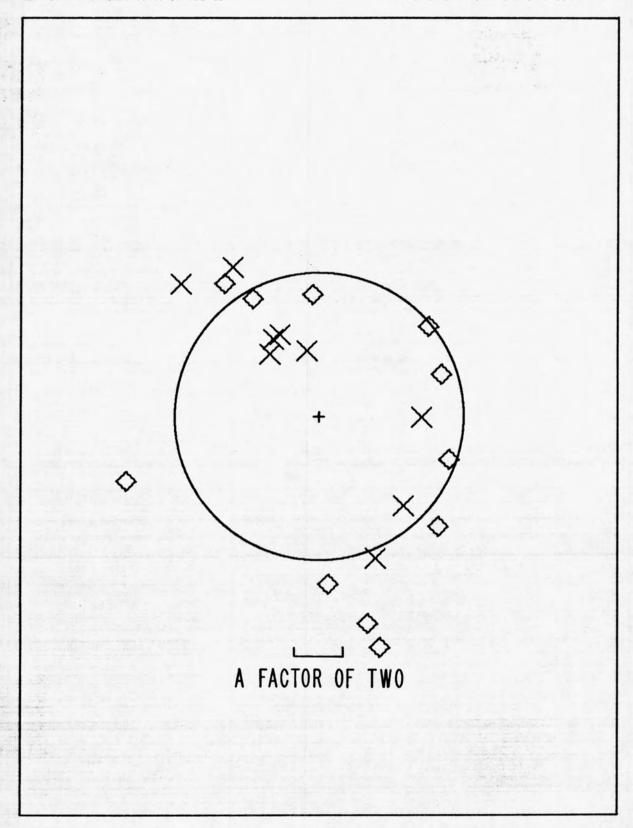


Figure 47. Proposed relationships for peak horizontal acceleration from a magnitude 6.6 earthquake. The curve labeled S is given by Schnabel and Seed (1973) for rock sites, the curve labeled D is given by Donovan (1973) for soil sites, and the curves labeled TO and T2 are the mean curves given by Trifunac (1976) for soft and hard sites respectively. The solid lines show the 70 percent prediction interval for the small-structure magnitude 6.0-6.4 data set of this report.

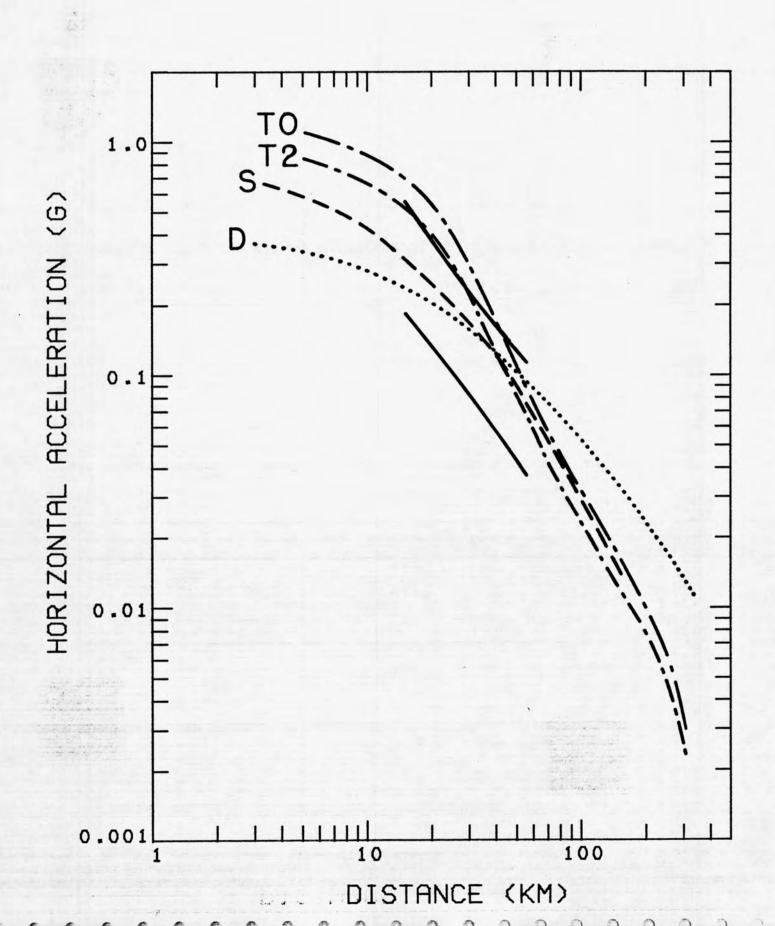


Figure 48. Proposed relationships for peak horizontal acceleration from a magnitude 7.6 earthquake. Curves labeled S, D, TO and T2 are from the sources given in Figure 41. The solid lines show the 70 percent prediction interval for the small-structure magnitude 7.1-7.7 data set of this report.

